

Julius Lee

RE: 368534 -

**1109 Coastal Bay Blvd.
Boynton Beach, FL 33435**

Site Information:

Project Customer: Dreambuilder Custom Homes Project Name: 368534 Model: 2347
Lot/Block: 6 Subdivision: The Sanctuary
Address:
City: Duval State: Florida

Name Address and License # of Structural Engineer of Record, If there is one, for the building.

Name: Pontigo, Luis Antonio, PE License #: 53311
Address: 420 Osceola Ave.
City: Jacksonville Beach State: Florida

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2007/TPI2002 Design Program: MiTek 20/20 7.3
Wind Code: ASCE 7-05 Wind Speed: 120 mph Floor Load: N/A psf
Roof Load: 32.0 psf

This package includes 54 individual, dated Truss Design Drawings and 0 Additional Drawings.
With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.
This document processed per section 16G15-23.003 of the Florida Board of Professionals Rules

In the event of changes from Builder or E.O.R. additional coversheets and drawings may accompany this coversheet. The latest approval dates supersede and replace the previous drawings.

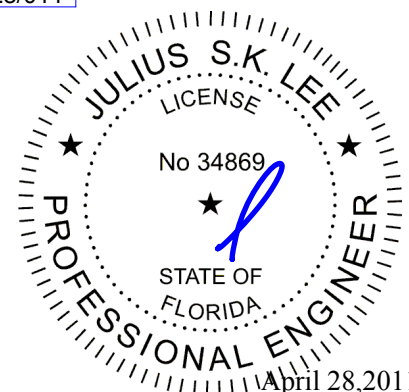
No.	Seal#	Truss Name	Date	No.	Seal#	Truss Name	Date
1	I4718505	CJ01	4/28/011	18	I4718522	HJ05	4/28/011
2	I4718506	CJ02	4/28/011	19	I4718523	T01	4/28/011
3	I4718507	CJ03	4/28/011	20	I4718524	T02	4/28/011
4	I4718508	CJ04	4/28/011	21	I4718525	T03	4/28/011
5	I4718509	CJ05	4/28/011	22	I4718526	T04	4/28/011
6	I4718510	CJ07	4/28/011	23	I4718527	T05	4/28/011
7	I4718511	CJ08	4/28/011	24	I4718528	T06	4/28/011
8	I4718512	CJ09	4/28/011	25	I4718529	T07	4/28/011
9	I4718513	EJ01	4/28/011	26	I4718530	T08	4/28/011
10	I4718514	EJ02	4/28/011	27	I4718531	T09	4/28/011
11	I4718515	EJ03	4/28/011	28	I4718532	T10	4/28/011
12	I4718516	EJ06	4/28/011	29	I4718533	T11	4/28/011
13	I4718517	EJ07	4/28/011	30	I4718534	T12	4/28/011
14	I4718518	HJ01	4/28/011	31	I4718535	T13	4/28/011
15	I4718519	HJ02	4/28/011	32	I4718536	T14	4/28/011
16	I4718520	HJ03	4/28/011	33	I4718537	T19	4/28/011
17	I4718521	HJ04	4/28/011	34	I4718538	T20	4/28/011

The truss drawing(s) referenced above have been prepared by MiTek Industries, Inc. under my direct supervision based on the parameters provided by Builders FirstSource (Jax).

Truss Design Engineer's Name: Julius Lee

My license renewal date for the state of Florida is February 28, 2013.

NOTE: The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI-1 Chapter 2.



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Site Information:

Project Customer: Dreambuilder Custom Homes Project Name: 368534 Model: 2347

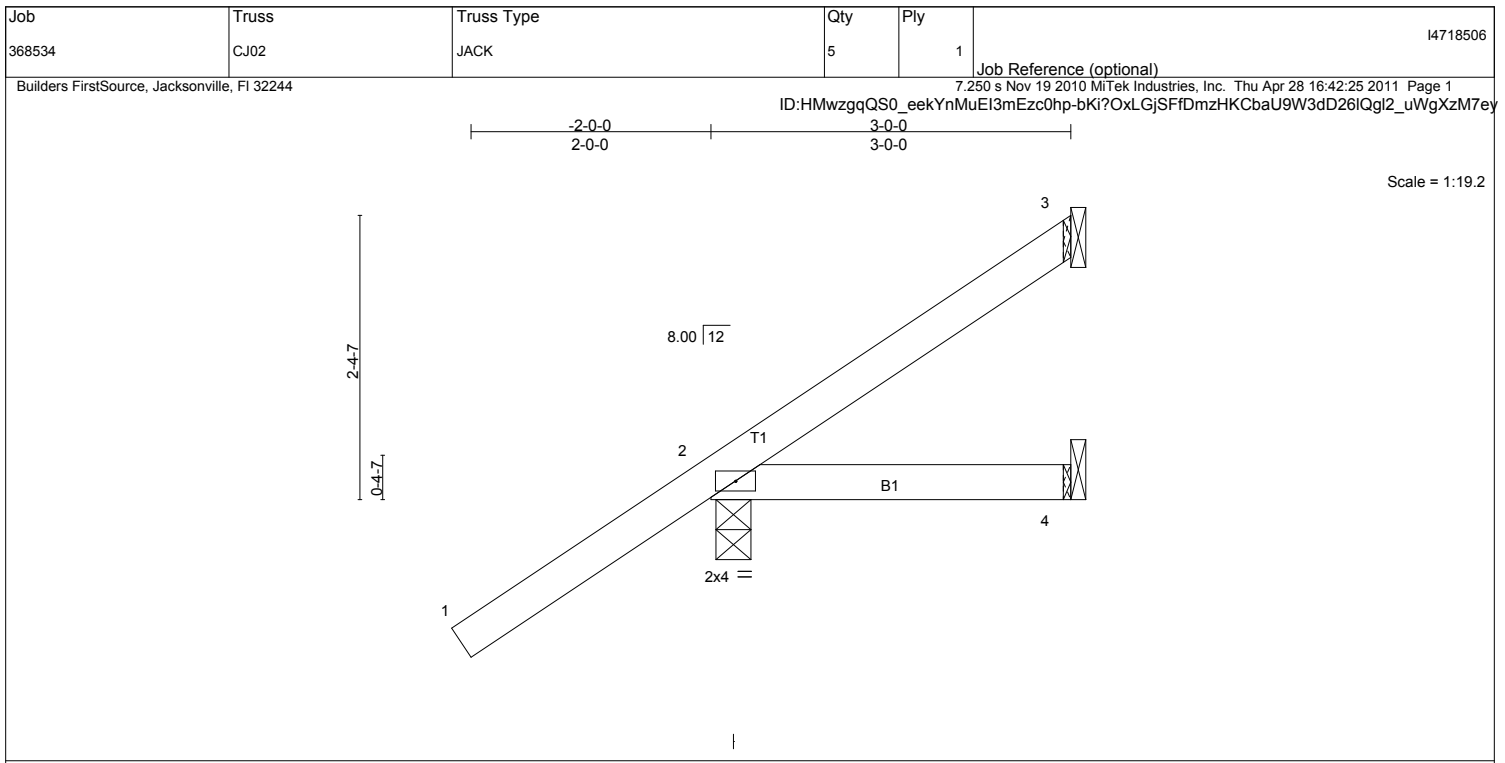
Lot/Block: 6 Subdivision: The Sanctuary

Address:

City: Duval

State: Florida

No.	Seal#	Truss Name	Date
35	I4718539	T21	4/28/011
36	I4718540	T22	4/28/011
37	I4718541	T23	4/28/011
38	I4718542	T24	4/28/011
39	I4718543	T25	4/28/011
40	I4718544	T26	4/28/011
41	I4718545	T27	4/28/011
42	I4718546	T28	4/28/011
43	I4718547	T29	4/28/011
44	I4718548	T30	4/28/011
45	I4718549	T31	4/28/011
46	I4718550	T32	4/28/011
47	I4718551	T33	4/28/011
48	I4718552	T34	4/28/011
49	I4718553	T35	4/28/011
50	I4718554	T36	4/28/011
51	I4718555	T37	4/28/011
52	I4718556	T38	4/28/011
53	I4718557	T39	4/28/011
54	I4718558	T40	4/28/011



LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.39	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.06	Vert(LL) -0.00 2-4 >999 240		
BCLL 0.0 *	Lumber Increase 1.25	WB 0.00	Vert(TL) -0.01 2-4 >999 180		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) -0.00 3 n/a n/a		
	Code FBC2007/TPI2002			Weight: 14 lb	FT = 20%

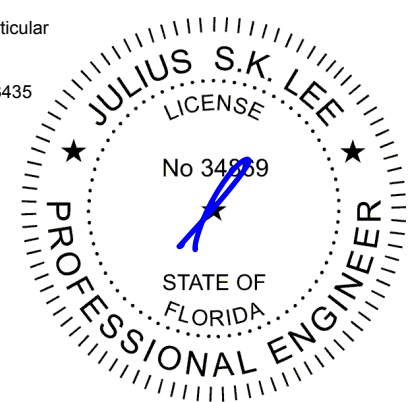
LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD 2 X 4 SYP No.2	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=27/Mechanical, 2=258/0-3-8, 4=14/Mechanical
Max Horz 2=213(LC 6)
Max Uplift 3=40(LC 7), 2=255(LC 6)
Max Grav 3=40(LC 4), 2=258(LC 1), 4=42(LC 2)

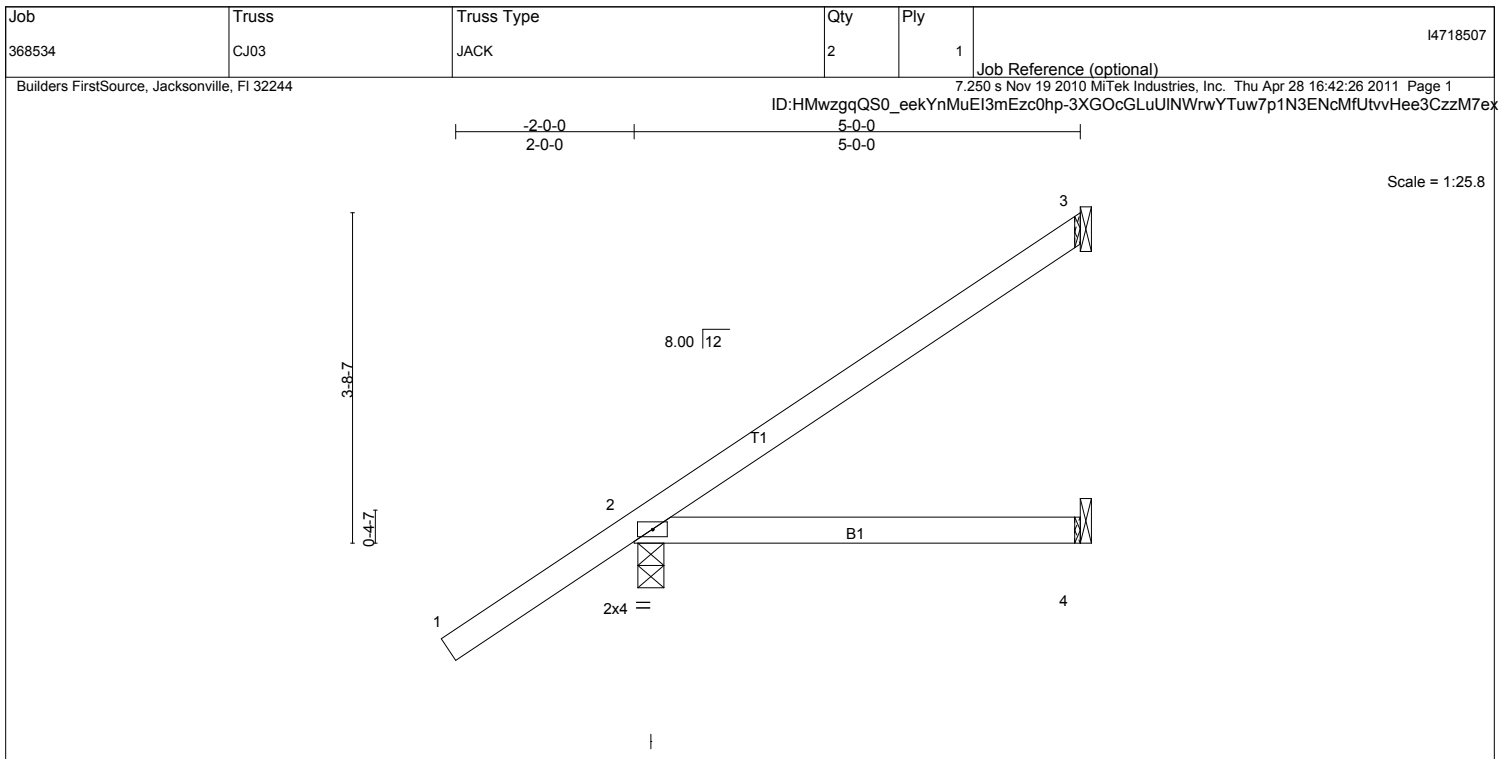
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- NOTES** (8-11)
- 1) Wind: ASCE 7-05; 120mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - 4) All bearings are assumed to be SYP No.2 .
 - 5) Refer to girder(s) for truss to truss connections.
 - 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 40 lb uplift at joint 3 and 255 lb uplift at joint 2.
 - 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - 9) For special connections with reactions or uplifts less than 300 lbs. Use typical toe-nail connection (refer to BFS detail package)
 - 10) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



April 28, 2011



LOADING (psf)	SPACING	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.39	Vert(LL) -0.03	2-4	>999	240	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.16	Vert(TL) -0.05	2-4	>999	180		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(TL) -0.00	3	n/a	n/a		
BCDL 5.0	Code FBC2007/TP12002	(Matrix)					Weight: 20 lb	FT = 20%

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD 2 X 4 SYP No.2	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

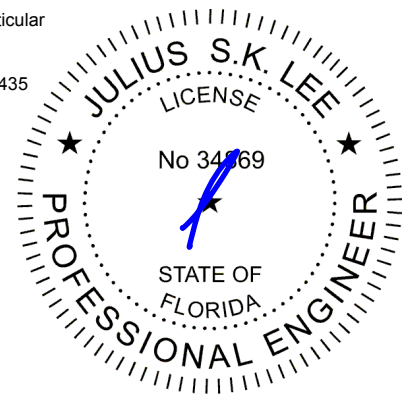
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 3=101/Mechanical, 2=302/0-3-8, 4=24/Mechanical
Max Horz 2=286(LC 6)
Max Uplift 3=125(LC 6), 2=235(LC 6)
Max Grav 3=101(LC 1), 2=302(LC 1), 4=72(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- NOTES** (8-11)
- 1) Wind: ASCE 7-05; 120mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - 4) All bearings are assumed to be SYP No.2 .
 - 5) Refer to girder(s) for truss to truss connections.
 - 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 125 lb uplift at joint 3 and 235 lb uplift at joint 2.
 - 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - 9) For special connections with reactions or uplifts less than 300 lbs. Use typical toe-nail connection (refer to BFS detail package)
 - 10) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



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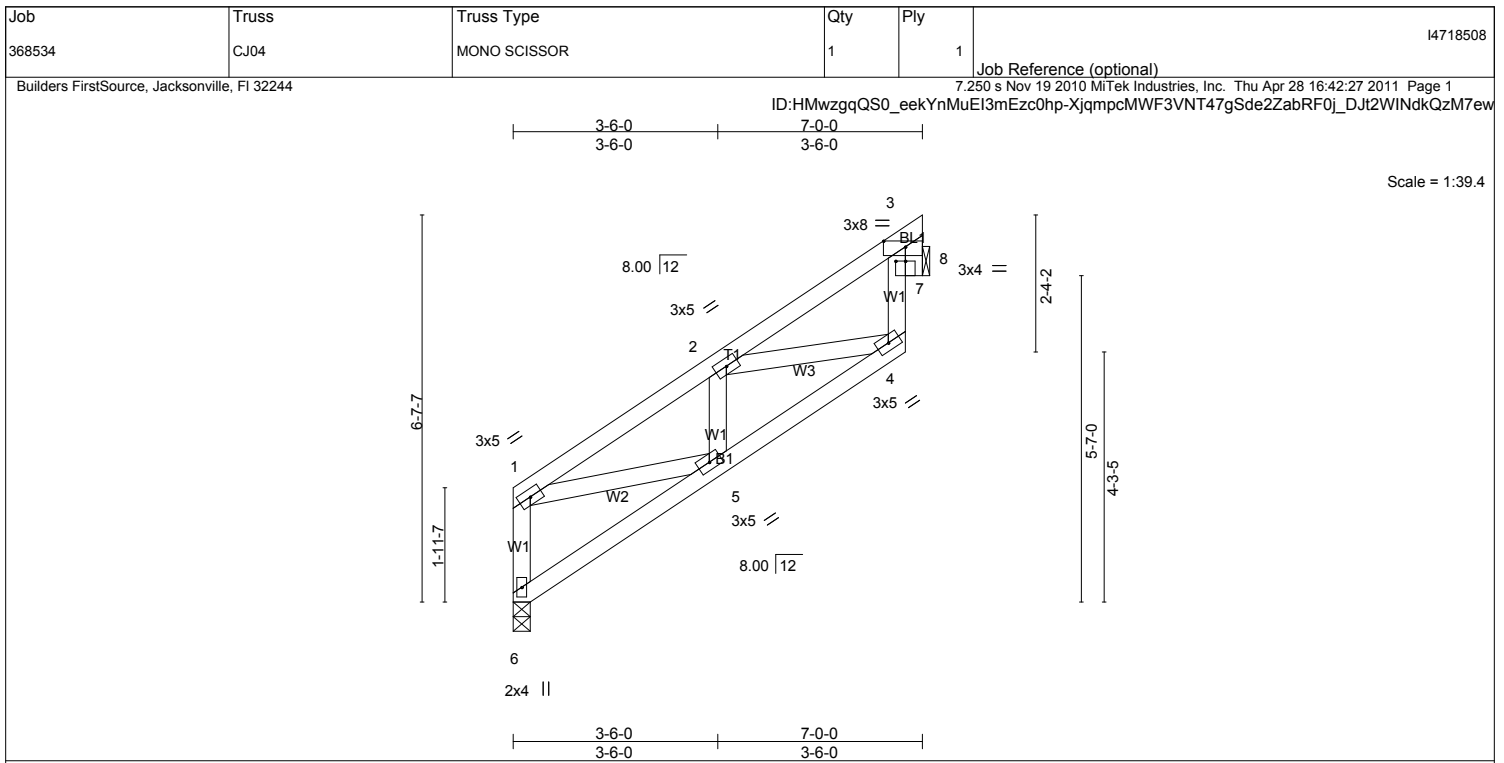


Plate Offsets (X,Y): [3:0-4-8,Edge], [7:0-2-0,0-0-0]					
LOADING (psf)	SPACING	CSI	DEFL	in (loc)	L/d
TCLL 20.0	Plates Increase 1.25	TC 0.26	Vert(LL) -0.01	5-6 >999	240
TCDL 7.0	Lumber Increase 1.25	BC 0.09	Vert(TL) -0.01	5-6 >999	180
BCLL 0.0 *	Rep Stress Incr YES	WB 0.08	Horz(TL) -0.01	8 n/a	n/a
BCDL 5.0	Code FBC2007/TPI2002	(Matrix)			
			PLATES	GRIP	
			MT20	244/190	
			Weight: 42 lb	FT = 20%	

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD 2 X 4 SYP No.2	BOT CHORD Rigid ceiling directly applied or 9-1-14 oc bracing.
WEBS 2 X 4 SYP No.3	
OTHERS 2 X 4 SYP No.3	

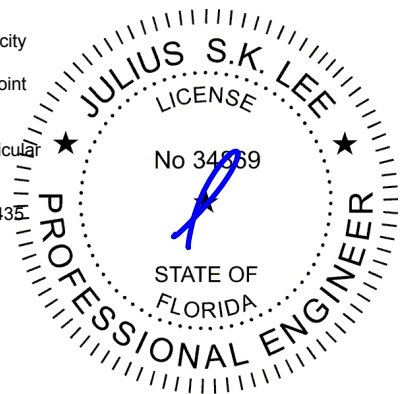
MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 6=217/0-3-8, 8=194/Mechanical
Max Horz 6=235(LC 6)
Max Uplift 6=1(LC 6), 8=259(LC 6)

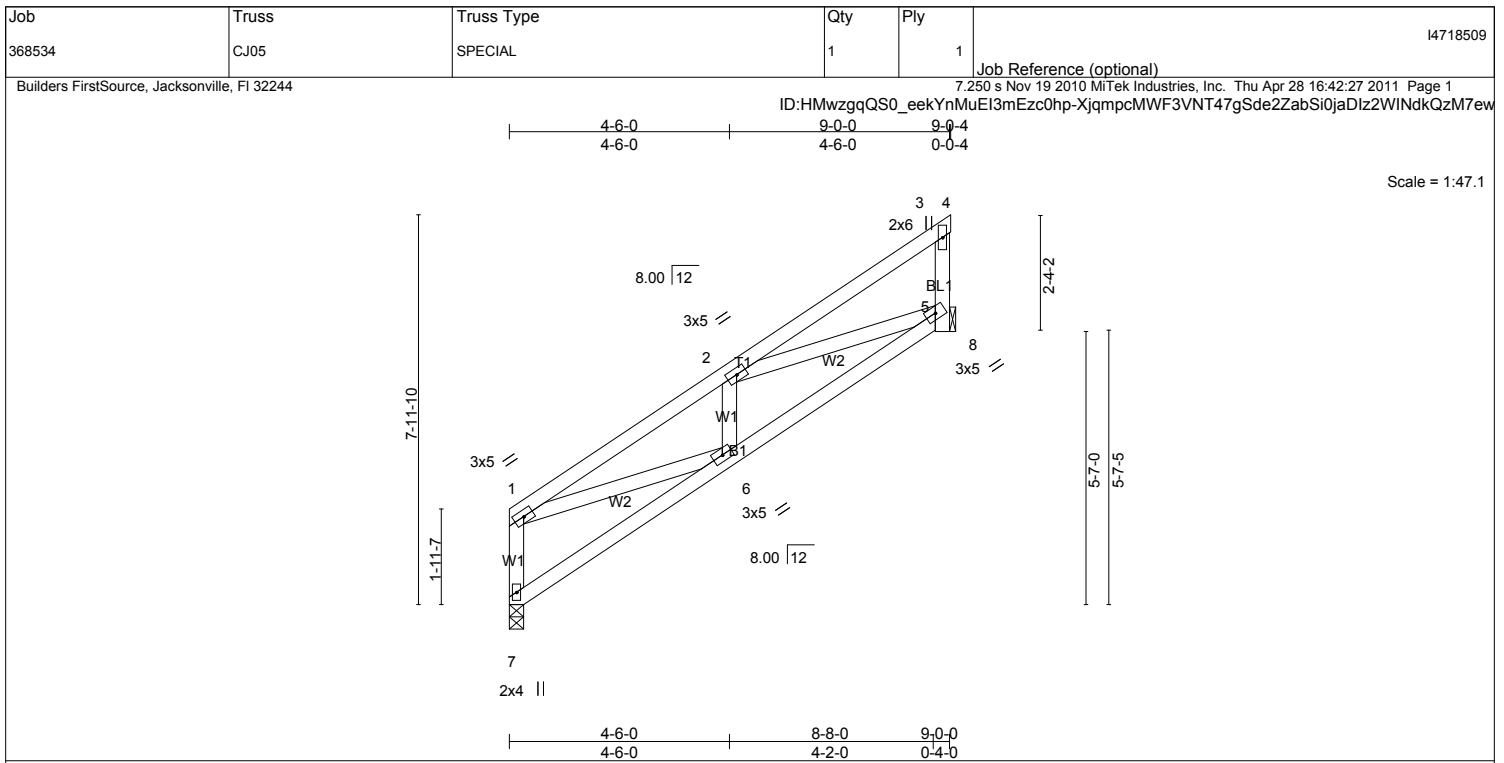
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=-320/241
BOT CHORD 5-6=-304/34, 4-5=-479/296
WEBS 2-4=-197/328

- NOTES** (9-12)
- 1) Wind: ASCE 7-05; 120mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - 4) All bearings are assumed to be SYP No.2 .
 - 5) Refer to girder(s) for truss to truss connections.
 - 6) Bearing at joint(s) 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 - 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1 lb uplift at joint 6 and 259 lb uplift at joint 8.
 - 8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - 10) For special connections with reactions or uplifts less than 300 lbs. Use typical toe-nail connection (refer to BFS detail package)
 - 11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

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LOADING (psf)	SPACING	CSI	DEFL	in (loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.23	Vert(LL) 0.02	6	>999	240	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.12	Vert(TL) -0.02	6-7	>999	180		
BCLL 0.0 *	Lumber Increase 1.25	WB 0.14	Horz(TL) -0.01	8	n/a	n/a		
BCDL 5.0	Rep Stress Incr YES	(Matrix)						
	Code FBC2007/TPI2002						Weight: 53 lb	FT = 20%

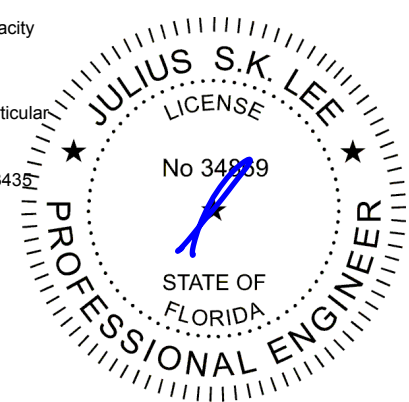
LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD 2 X 4 SYP No.2	BOT CHORD Rigid ceiling directly applied or 7-6-1 oc bracing.
WEBS 2 X 4 SYP No.3	
OTHERS 2 X 4 SYP No.3	

REACTIONS (lb/size) 7=279/0-3-8, 8=281/Mechanical
Max Horz 7=324(LC 6)
Max Uplift 8=360(LC 6)

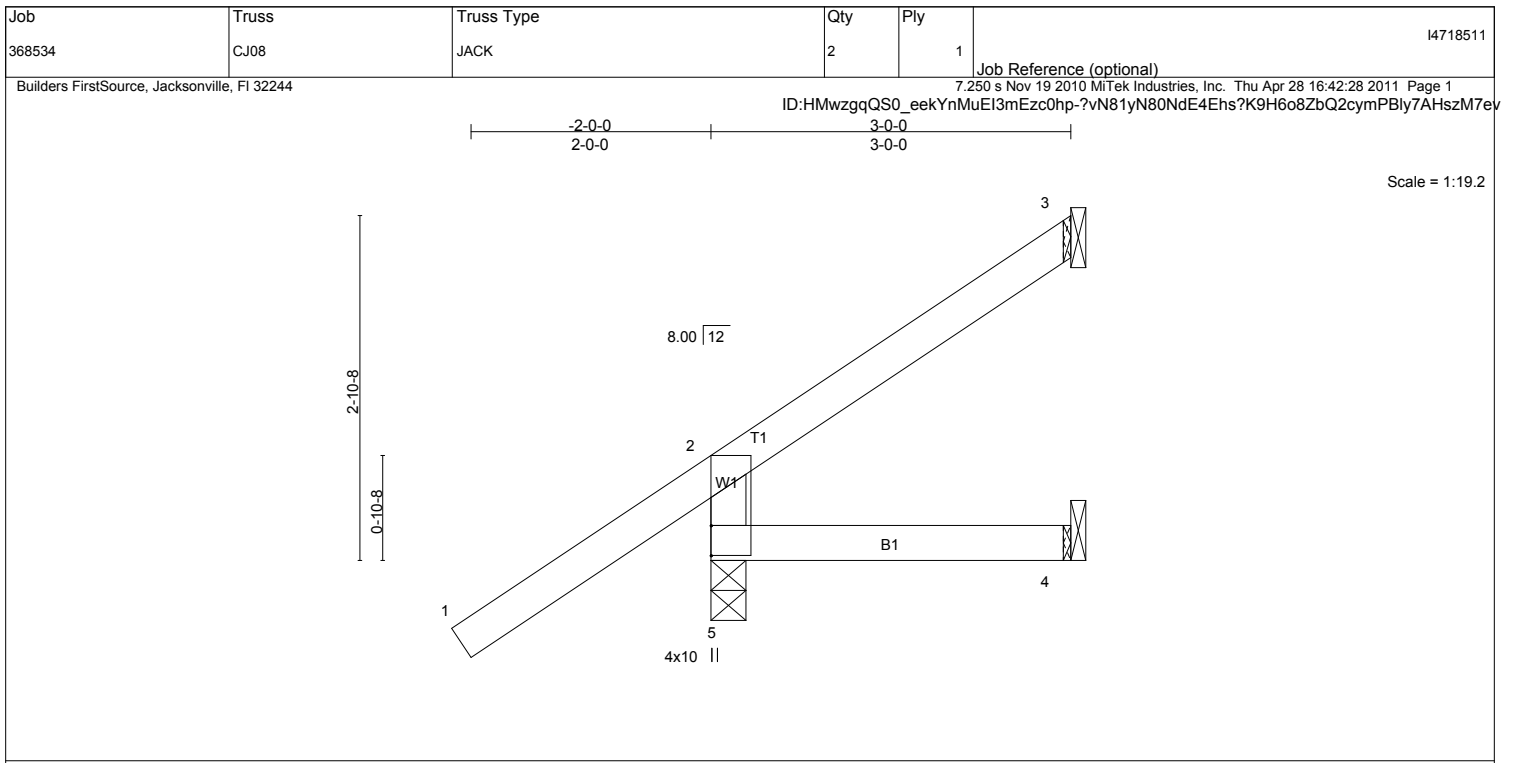
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-7=-273/249, 1-2=-531/398, 5-8=-281/361
BOT CHORD 6-7=-428/62, 5-6=-709/487
WEBS 1-6=-233/379, 2-5=-368/554

- NOTES** (9-12)
- 1) Wind: ASCE 7-05; 120mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - 4) All bearings are assumed to be SYP No.2 .
 - 5) Refer to girder(s) for truss to truss connections.
 - 6) Bearing at joint(s) 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 - 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 360 lb uplift at joint 8.
 - 8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - 10) For special connections with reactions or uplifts less than 300 lbs. Use typical toe-nail connection (refer to BFS detail package)
 - 11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

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LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.47	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.07	Vert(LL) -0.00 4-5 >999 240		
BCLL 0.0 *	Lumber Increase 1.25	WB 0.00	Vert(TL) -0.00 4-5 >999 180		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) -0.01 3 n/a n/a		
	Code FBC2007/TP12002			Weight: 14 lb	FT = 20%

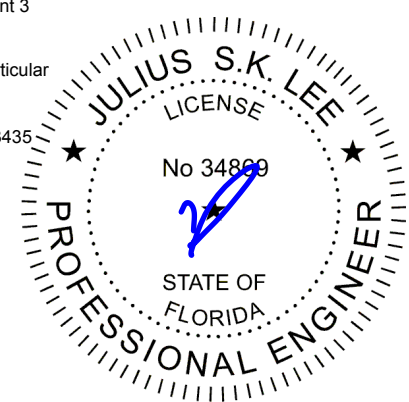
LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins, except end verticals.
BOT CHORD 2 X 4 SYP No.2	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 2 X 4 SYP No.3	
	MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 5=258/0-3-8, 3=40/Mechanical, 4=1/Mechanical
Max Horz 5=216(LC 6)
Max Uplift 5=-216(LC 6), 3=-57(LC 6), 4=-1(LC 7)
Max Grav 5=258(LC 1), 3=40(LC 1), 4=35(LC 2)

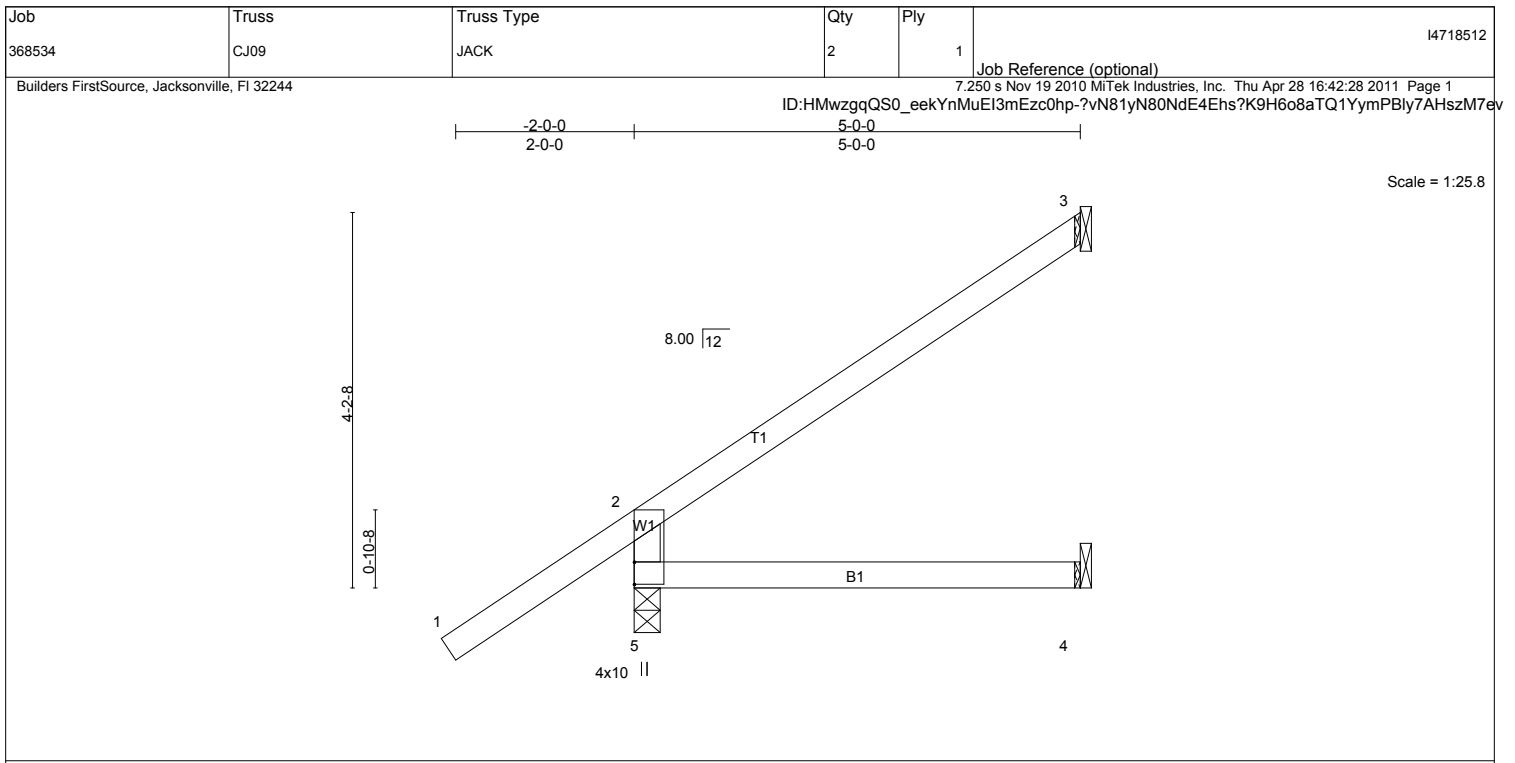
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-5=-231/260

- NOTES** (8-11)
- 1) Wind: ASCE 7-05; 120mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - 4) All bearings are assumed to be SYP No.2.
 - 5) Refer to girder(s) for truss to truss connections.
 - 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 216 lb uplift at joint 5, 57 lb uplift at joint 3 and 1 lb uplift at joint 4.
 - 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TP1 1 as referenced by the building code.
 - 9) For special connections with reactions or uplifts less than 300 lbs. Use typical toe-nail connection (refer to BFS detail package)
 - 10) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

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LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase	1.25	TC 0.42	Vert(LL)	0.03	4-5	>999	240	MT20
TCDL 7.0	Lumber Increase	1.25	BC 0.14	Vert(TL)	-0.04	4-5	>999	180	244/190
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.00	Horz(TL)	-0.02	3	n/a	n/a	
BCDL 5.0	Code FBC2007/TP12002		(Matrix)						
								Weight: 21 lb	FT = 20%

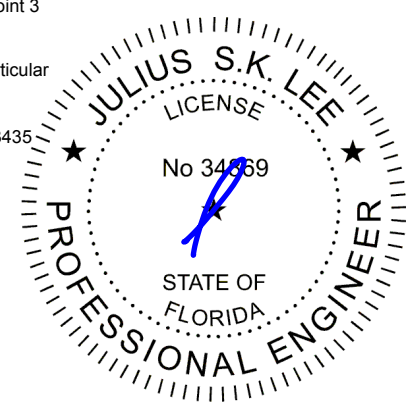
LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins, except end verticals.
BOT CHORD 2 X 4 SYP No.2	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 2 X 4 SYP No.3	
	MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 5=302/0-3-8, 3=102/Mechanical, 4=24/Mechanical
Max Horz 5=289(LC 6)
Max Uplift 5=205(LC 6), 3=138(LC 6), 4=-4(LC 7)
Max Grav 5=302(LC 1), 3=102(LC 1), 4=66(LC 2)

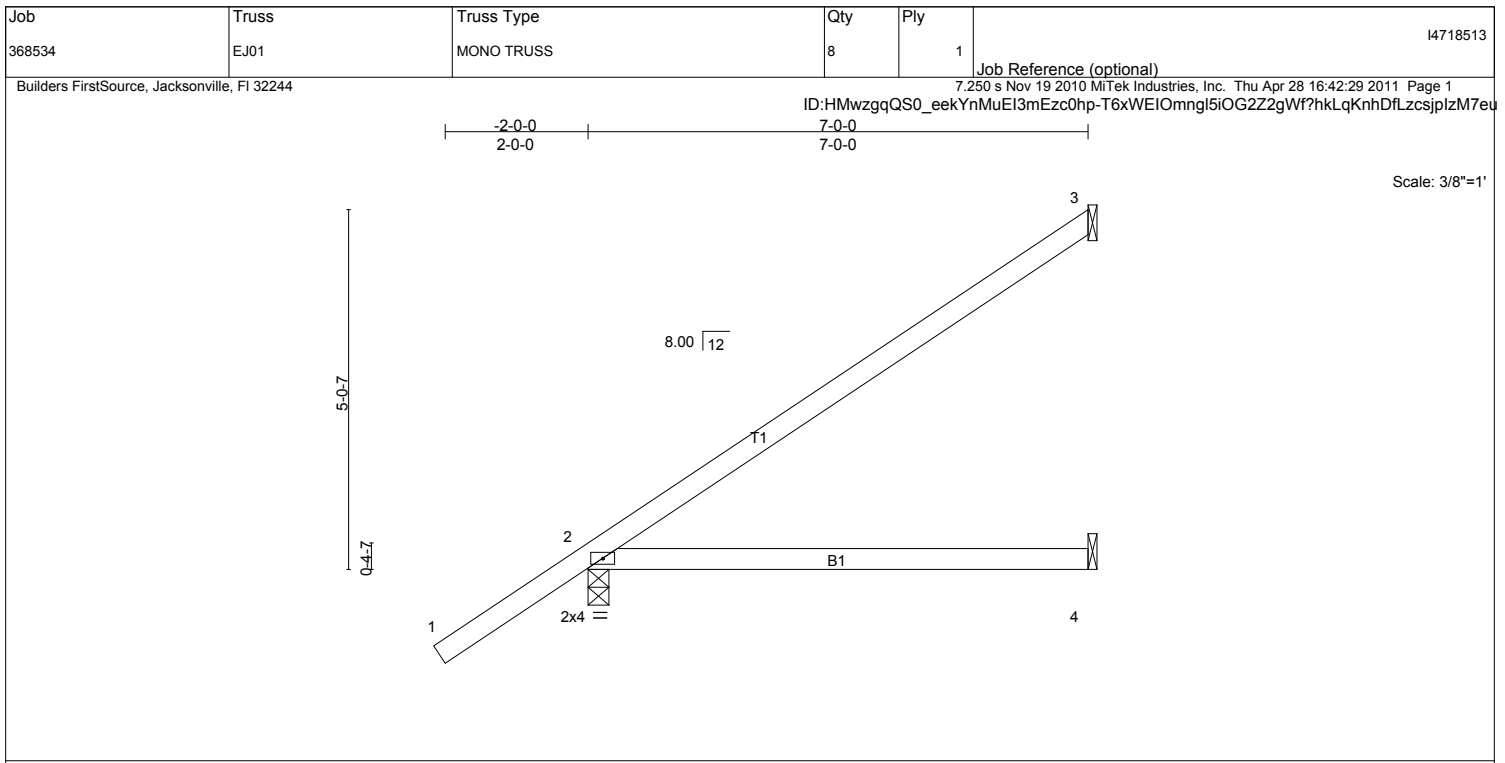
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-5=-277/273

- NOTES** (8-11)
- 1) Wind: ASCE 7-05; 120mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - 4) All bearings are assumed to be SYP No.2.
 - 5) Refer to girder(s) for truss to truss connections.
 - 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 205 lb uplift at joint 5, 138 lb uplift at joint 3 and 4 lb uplift at joint 4.
 - 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - 9) For special connections with reactions or uplifts less than 300 lbs. Use typical toe-nail connection (refer to BFS detail package)
 - 10) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



April 28, 2011



LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.47	Vert(LL)	-0.11	2-4	>730	240	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.33	Vert(TL)	-0.20	2-4	>417	180		
BCLL 0.0 *	Lumber Increase 1.25	WB 0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL 5.0	Rep Stress Incr YES	(Matrix)							
	Code FBC2007/TP12002							Weight: 27 lb	FT = 20%

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD 2 X 4 SYP No.2	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

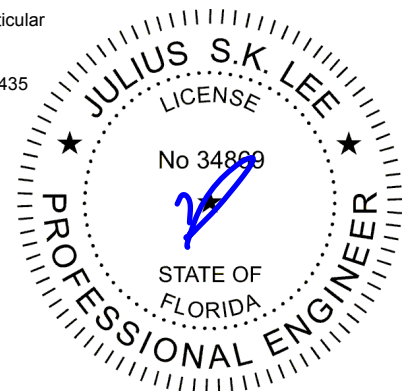
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 3=164/Mechanical, 2=357/0-3-8, 4=34/Mechanical
Max Horz 2=259(LC 6)
Max Uplift 3=136(LC 6), 2=161(LC 6)
Max Grav 3=164(LC 1), 2=357(LC 1), 4=102(LC 2)

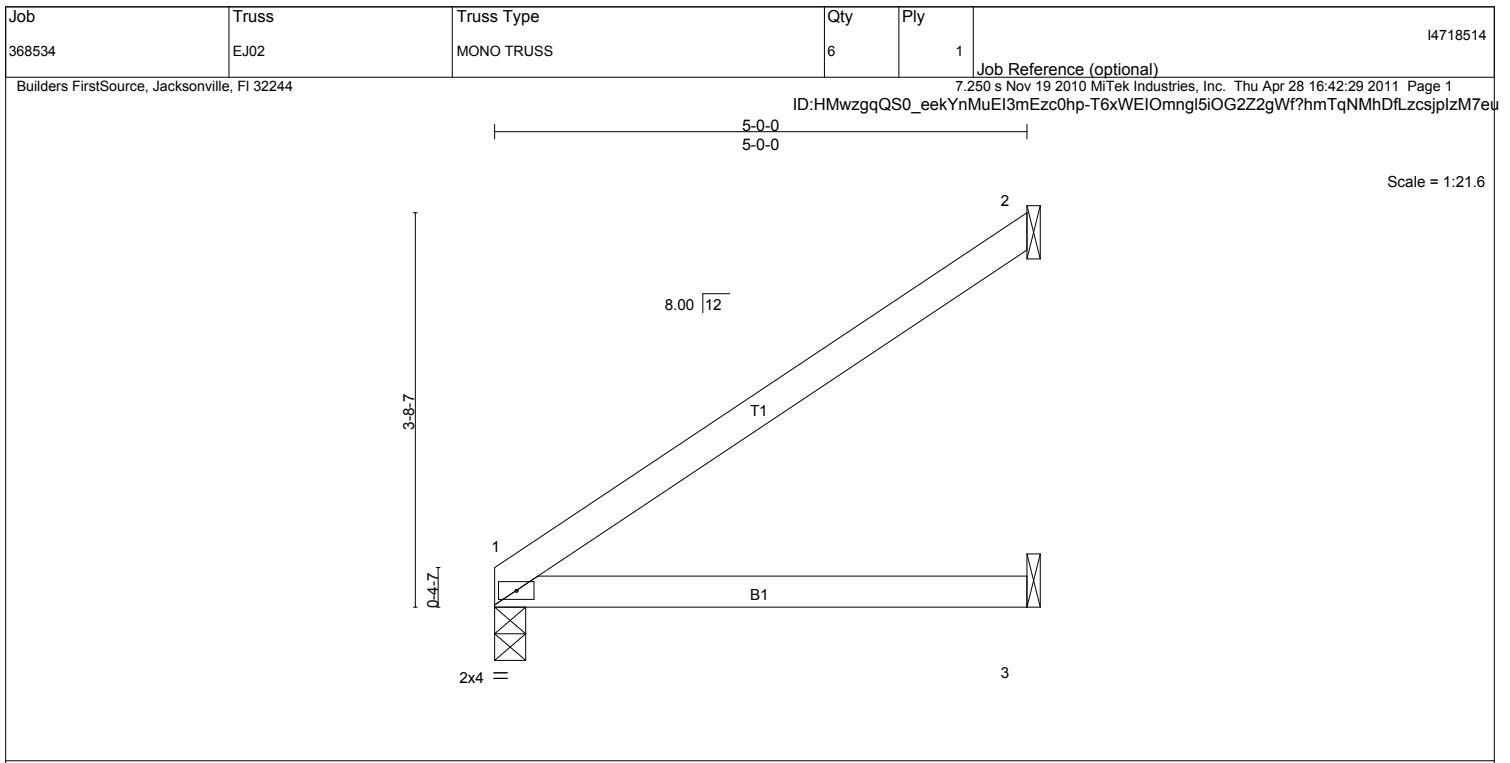
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- NOTES** (8-11)
- Wind: ASCE 7-05; 120mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - All bearings are assumed to be SYP No.2 .
 - Refer to girder(s) for truss to truss connections.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 136 lb uplift at joint 3 and 161 lb uplift at joint 2.
 - "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - For special connections with reactions or uplifts less than 300 lbs. Use typical toe-nail connection (refer to BFS detail package)
 - Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

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LOADING (psf)	SPACING	CSI	DEFL	in (loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.34	Vert(LL)	-0.03	1-3	>999	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.16	Vert(TL)	-0.05	1-3	>999		
BCLL 0.0 *	Lumber Increase 1.25	WB 0.00	Horz(TL)	-0.00	2	n/a		
BCDL 5.0	Rep Stress Incr YES	(Matrix)						
	Code FBC2007/TP12002						Weight: 17 lb	FT = 20%

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins.
BOT CHORD 2 X 4 SYP No.2	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

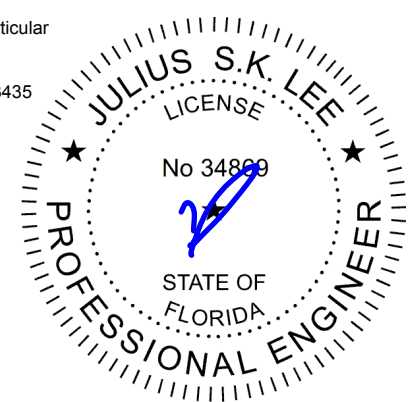
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 1=153/0-3-8, 2=129/Mechanical, 3=24/Mechanical
Max Horz 1=183(LC 6)
Max Uplift 1=-33(LC 6), 2=-175(LC 6)
Max Grav 1=153(LC 1), 2=129(LC 1), 3=72(LC 2)

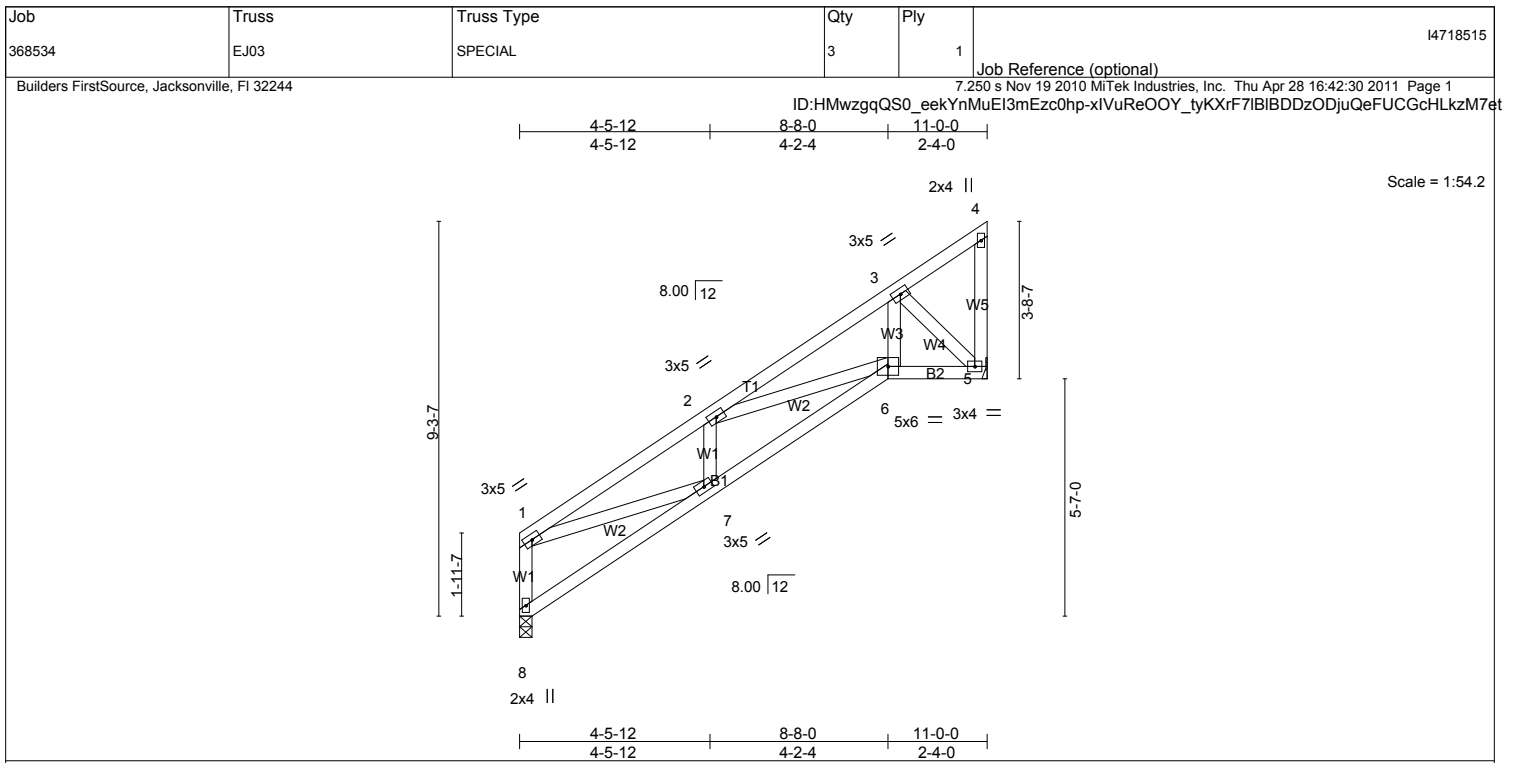
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- NOTES** (8-11)
- 1) Wind: ASCE 7-05; 120mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - 4) All bearings are assumed to be SYP No.2 .
 - 5) Refer to girder(s) for truss to truss connections.
 - 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 33 lb uplift at joint 1 and 175 lb uplift at joint 2.
 - 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - 9) For special connections with reactions or uplifts less than 300 lbs. Use typical toe-nail connection (refer to BFS detail package)
 - 10) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

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LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.20	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.14	Vert(LL) 0.04 7 >999 240		
BCLL 0.0 *	Lumber Increase 1.25	WB 0.17	Vert(TL) -0.04 6-7 >999 180		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) -0.03 5 n/a n/a		
	Code FBC2007/TPI2002			Weight: 67 lb	FT = 20%

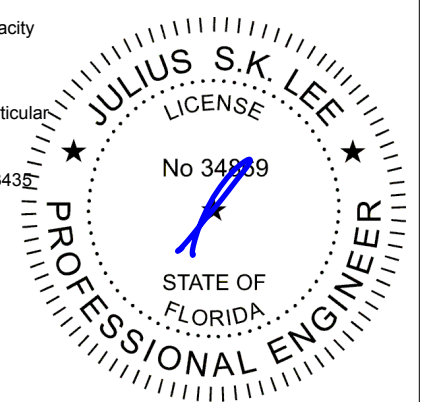
LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD 2 X 4 SYP No.2	BOT CHORD Rigid ceiling directly applied or 6-5-10 oc bracing.
WEBS 2 X 4 SYP No.3	

REACTIONS (lb/size) 8=343/0-3-8, 5=343/Mechanical
Max Horz 8=273(LC 6)
Max Uplift 5=264(LC 6)

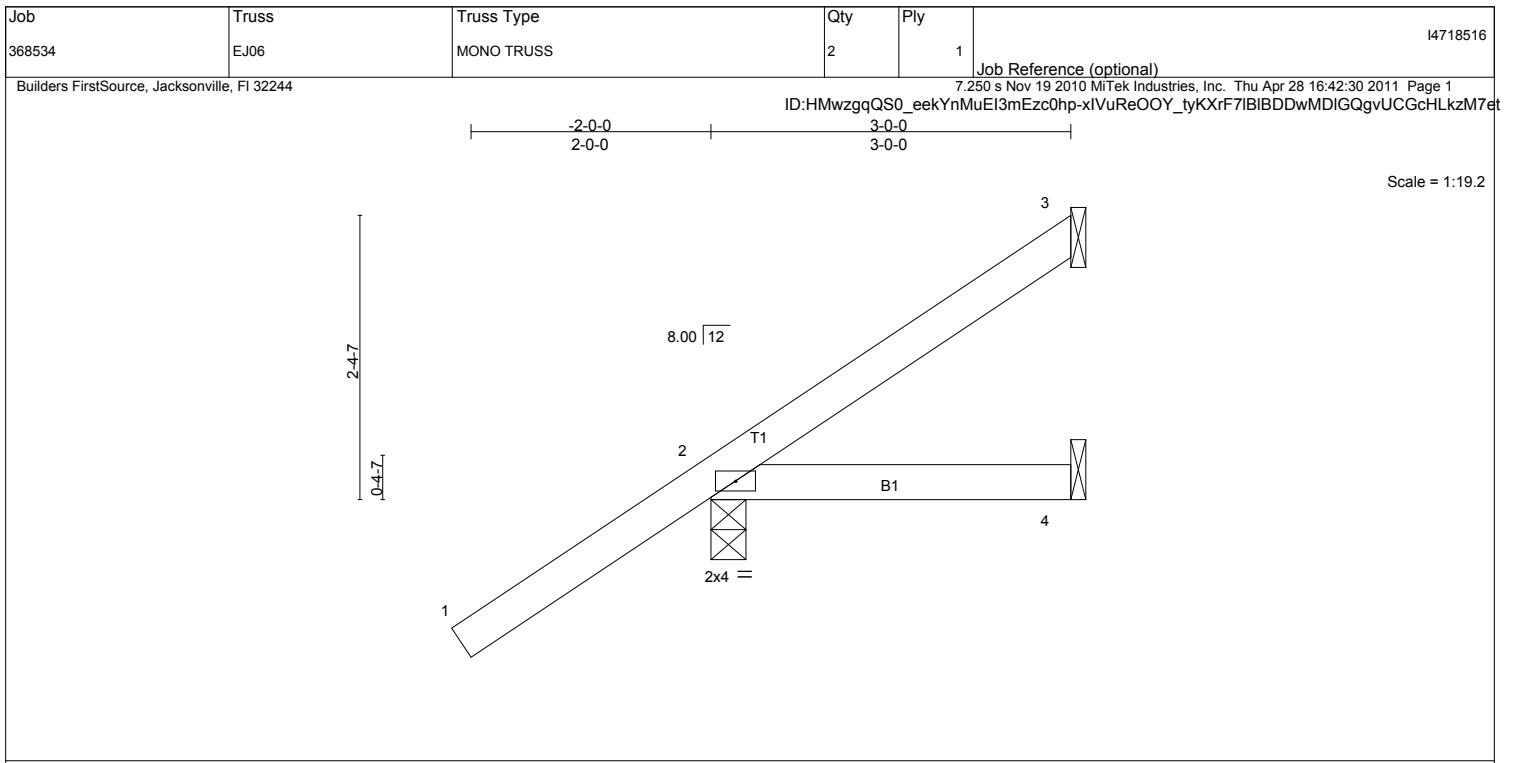
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-8=-338/307, 1-2=-712/571, 2-3=-495/364
BOT CHORD 7-8=-506/62, 6-7=-958/662, 5-6=-412/328
WEBS 1-7=-382/533, 2-6=-195/354, 3-6=-429/351, 3-5=-448/565

- NOTES** (9-13)
- 1) Wind: ASCE 7-05; 120mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - 4) All bearings are assumed to be SYP No.2 .
 - 5) Refer to girder(s) for truss to truss connections.
 - 6) Bearing at joint(s) 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 - 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 264 lb uplift at joint 5.
 - 8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - 10) For special connections with reactions or uplifts less than 300 lbs. Use typical toe-nail connection (refer to BFS detail package)
 - 11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435
 - 12) Use Simpson HTU26 to attach Truss to Carrying member

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LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.39	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.06	Vert(LL) -0.00 2-4 >999 240		
BCLL 0.0 *	Lumber Increase 1.25	WB 0.00	Vert(TL) -0.01 2-4 >999 180		
BCDL 5.0	Rep Stress Incr YES	(Matrix)	Horz(TL) -0.00 3 n/a n/a		
	Code FBC2007/TPI2002			Weight: 14 lb	FT = 20%

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 3-0-0 oc purlins.
BOT CHORD 2 X 4 SYP No.2	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

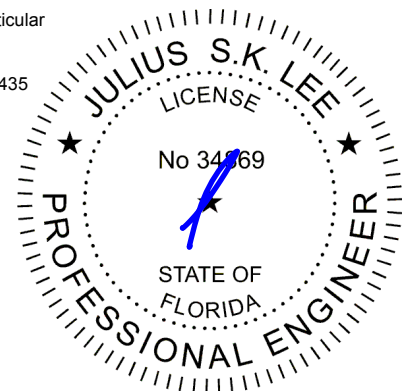
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 3=27/Mechanical, 2=258/0-3-8, 4=14/Mechanical
Max Horz 2=213(LC 6)
Max Uplift 3=40(LC 7), 2=255(LC 6)
Max Grav 3=40(LC 4), 2=258(LC 1), 4=42(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- NOTES** (8-11)
- 1) Wind: ASCE 7-05; 120mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - 4) All bearings are assumed to be SYP No.2 .
 - 5) Refer to girder(s) for truss to truss connections.
 - 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 40 lb uplift at joint 3 and 255 lb uplift at joint 2.
 - 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - 9) For special connections with reactions or uplifts less than 300 lbs. Use typical toe-nail connection (refer to BFS detail package)
 - 10) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

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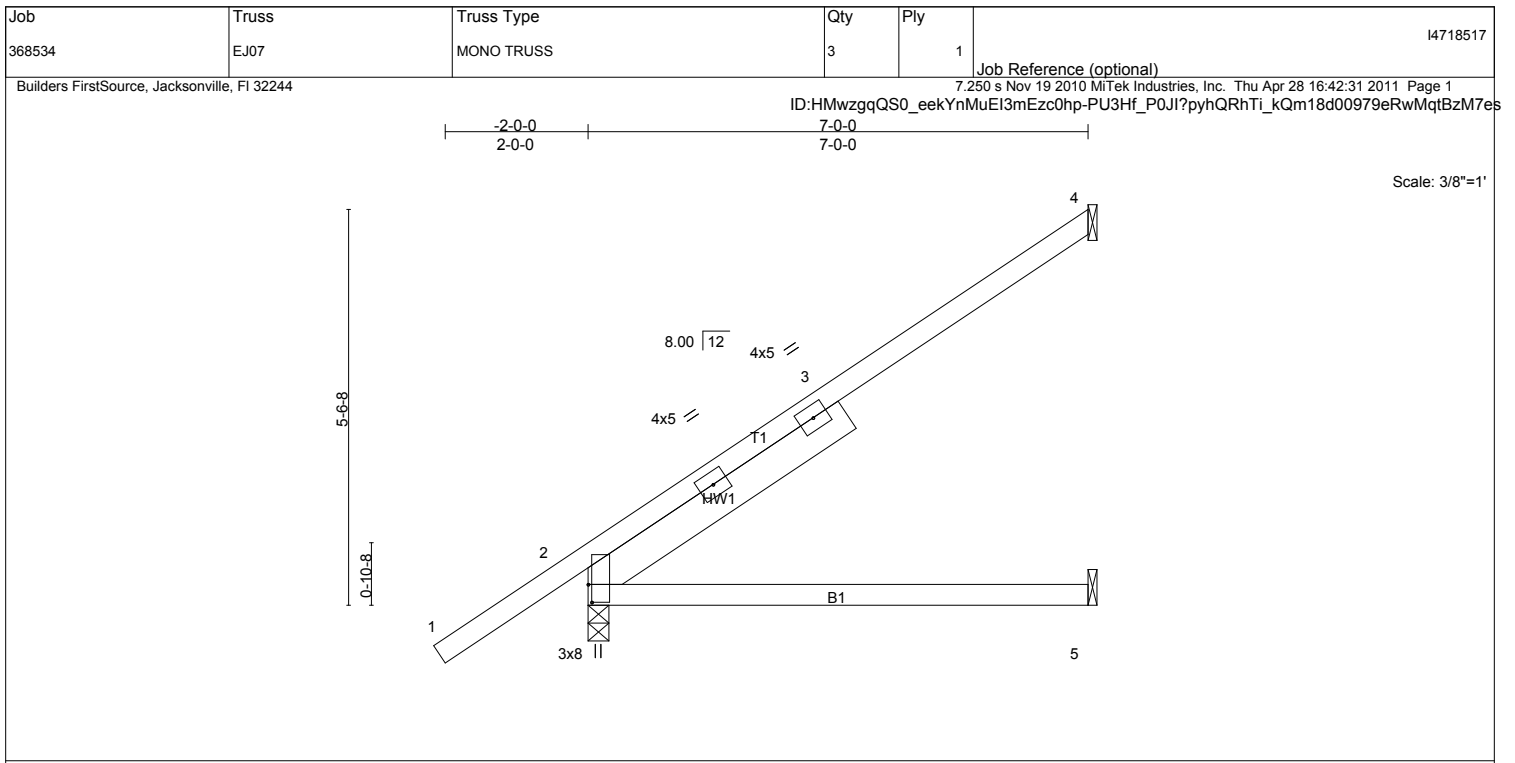


Plate Offsets (X,Y): [2:0-3-0,0-0-10]												
LOADING (psf)		SPACING 2-0-0		CSI		DEFL in (loc) l/defl L/d				PLATES GRIP		
TCLL	20.0	Plates Increase	1.25	TC	0.65	Vert(LL)	-0.12	2-5	>685	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.34	Vert(TL)	-0.21	2-5	>391	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.05	4	n/a	n/a		
BCDL	5.0	Code FBC2007/TPI2002		(Matrix)							Weight: 37 lb	FT = 20%

LUMBER
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
SLIDER Left 2 X 6 SYP No.2 4-4-0

BRACING
TOP CHORD
BOT CHORD
Structural wood sheathing directly applied or 6-0-0 oc purlins.
Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

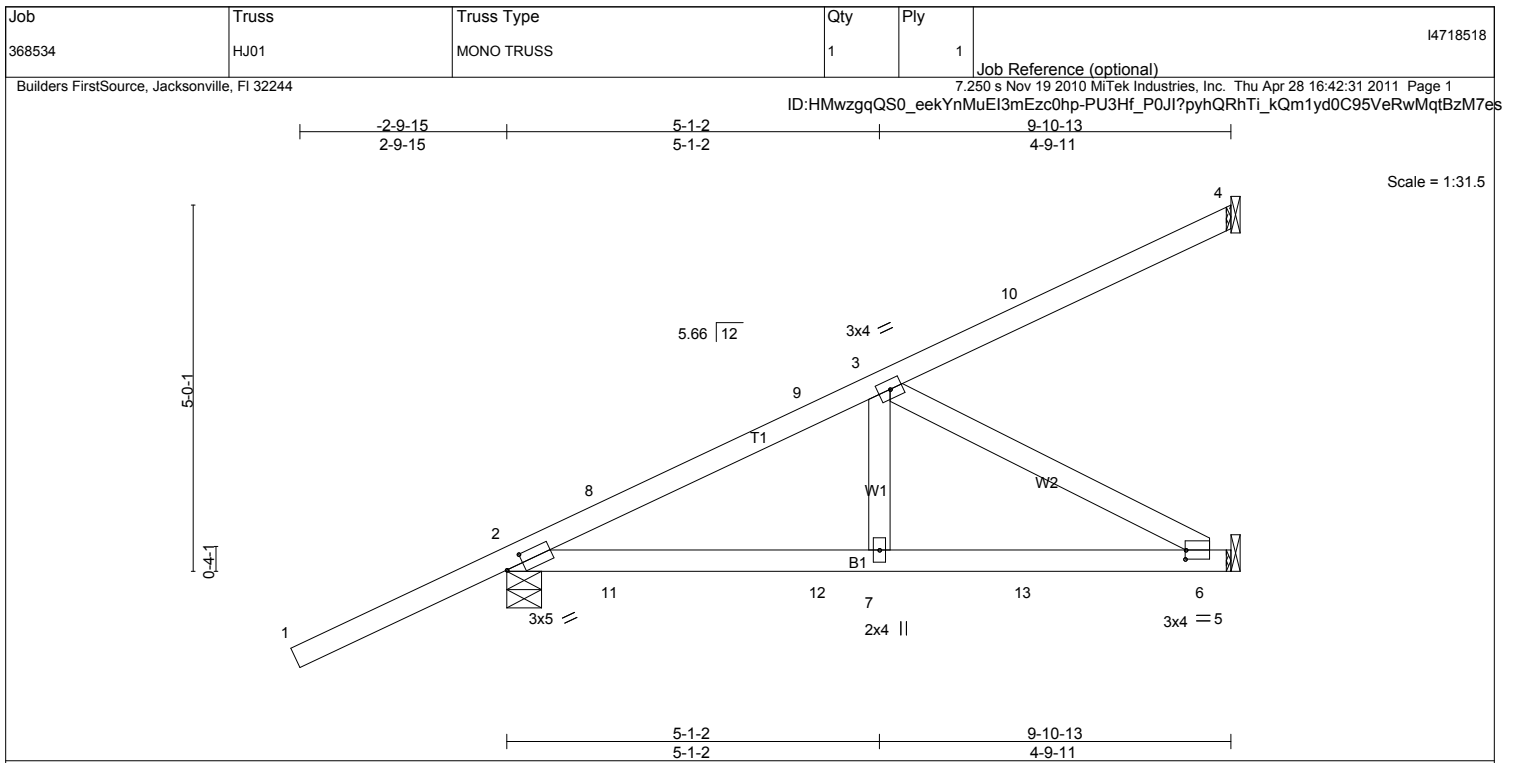
REACTIONS (lb/size) 4=170/Mechanical, 2=351/0-3-8, 5=35/Mechanical
Max Horz 2=248(LC 6)
Max Uplift 4=-158(LC 6), 2=-136(LC 6)
Max Grav 4=170(LC 1), 2=351(LC 1), 5=104(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES (8-11)
1) Wind: ASCE 7-05; 120mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
4) All bearings are assumed to be SYP No.2 .
5) Refer to girder(s) for truss to truss connections.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 158 lb uplift at joint 4 and 136 lb uplift at joint 2.
7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
9) For special connections with reactions or uplifts less than 300 lbs. Use typical toe-nail connection (refer to BFS detail package)
10) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

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LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.66	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.33	Vert(LL) -0.04 6-7 >999 240		
BCLL 0.0 *	Lumber Increase 1.25	WB 0.17	Vert(TL) -0.08 6-7 >999 180		
BCDL 5.0	Rep Stress Incr NO	(Matrix)	Horz(TL) -0.01 5 n/a n/a		
	Code FBC2007/TPI2002			Weight: 46 lb	FT = 20%

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD 2 X 4 SYP No.2	BOT CHORD Rigid ceiling directly applied or 9-11-9 oc bracing.
WEBS 2 X 4 SYP No.3	

REACTIONS (lb/size) 4=146/Mechanical, 2=434/0-5-11, 5=191/Mechanical
Max Horz 2=358(LC 5)
Max Uplift 4=180(LC 5), 2=493(LC 5), 5=174(LC 5)
Max Grav 4=146(LC 1), 2=434(LC 1), 5=230(LC 2)

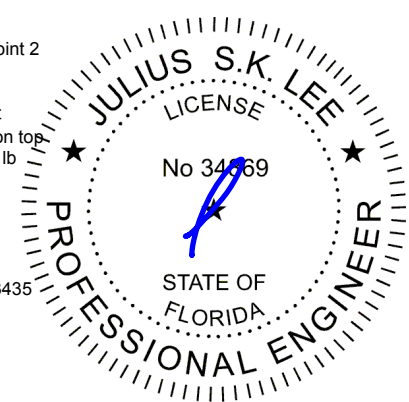
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-8=-379/267, 8-9=-385/259, 3-9=-333/240
BOT CHORD 2-11=-367/309, 11-12=-367/309, 7-12=-367/309, 7-13=-367/309, 6-13=-367/309
WEBS 3-6=-352/417

- NOTES** (10-13)
- 1) Wind: ASCE 7-05; 120mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60
 - 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - 4) All bearings are assumed to be SYP No.2.
 - 5) Refer to girder(s) for truss to truss connections.
 - 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 180 lb uplift at joint 4, 493 lb uplift at joint 2 and 174 lb uplift at joint 5.
 - 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - 8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 40 lb up at 1-5-12, 40 lb up at 1-5-12, 31 lb up at 4-3-11, 31 lb up at 4-3-11, and 47 lb down and 116 lb up at 7-1-10, and 47 lb down and 116 lb up at 7-1-10 on top chord, and 16 lb up at 1-5-12, 16 lb up at 1-5-12, 12 lb down at 4-3-11, 12 lb down at 4-3-11, and 42 lb down at 7-1-10, and 42 lb down at 7-1-10 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
 - 9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
 - 10) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - 11) For special connections with reactions or uplifts less than 300 lbs. Use typical toe-nail connection (refer to BFS detail package)
 - 12) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard
1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Continued on page 2

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Job	Truss	Truss Type	Qty	Ply	
368534	HJ01	MONO TRUSS	1	1	l4718518

Builders FirstSource, Jacksonville, Fl 32244

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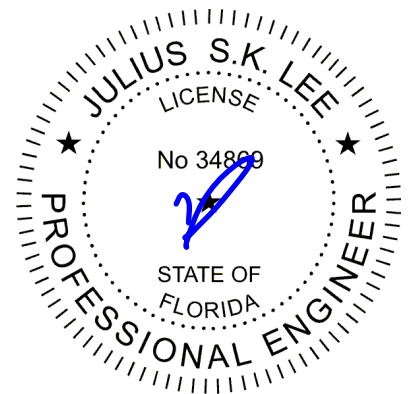
LOAD CASE(S) Standard

Uniform Loads (plf)

Vert: 1-4=-54, 2-5=-10

Concentrated Loads (lb)

Vert: 8=80(F=40, B=40) 9=53(F=27, B=27) 10=-95(F=-47, B=-47) 11=10(F=5, B=5) 12=-8(F=-4, B=-4) 13=-28(F=-14, B=-14)



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WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

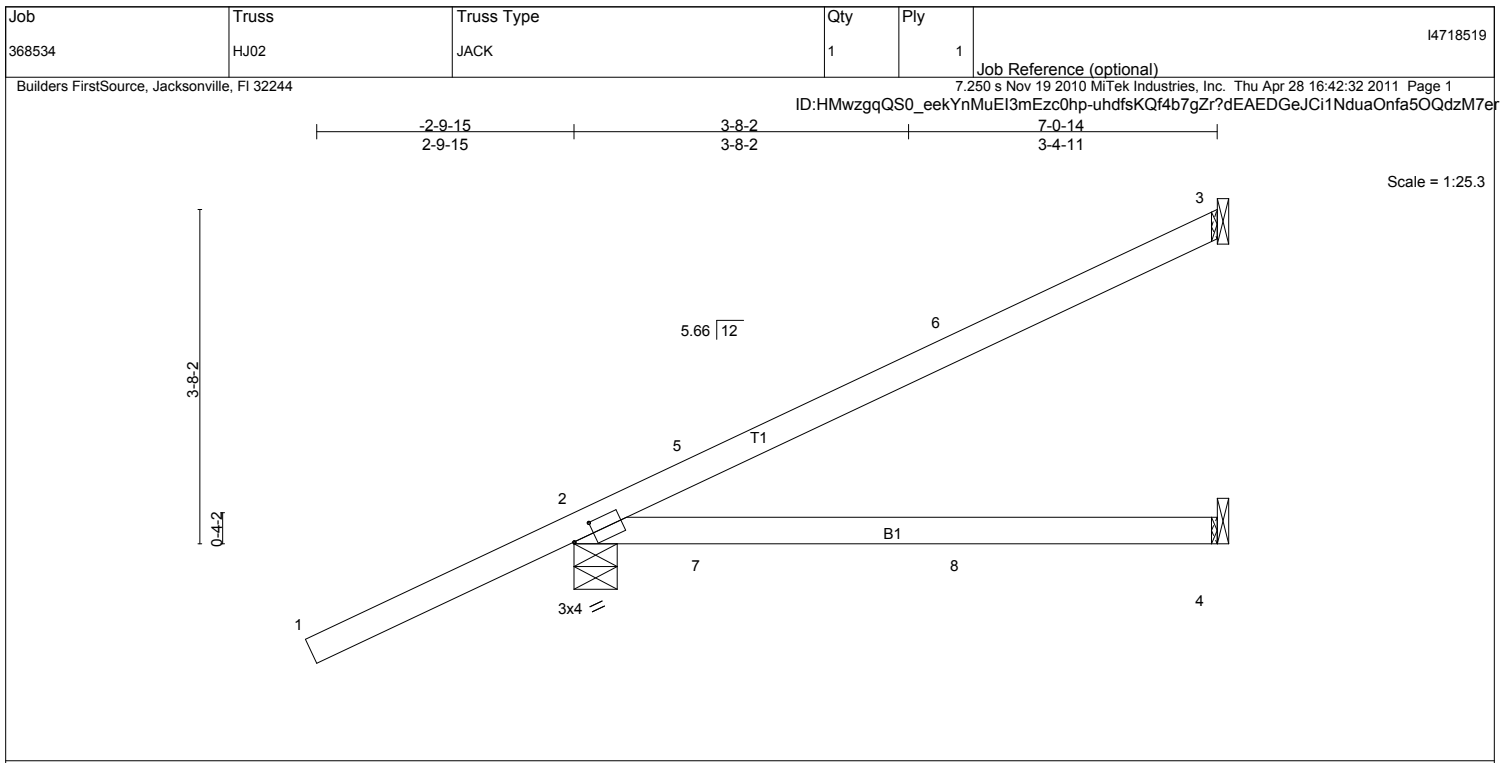


Plate Offsets (X,Y): [2:0-2-13,0-1-8]												
LOADING (psf)		SPACING 2-0-0		CSI		DEFL in (loc) l/defl L/d			PLATES	GRIP		
TCLL	20.0	Plates Increase	1.25	TC	0.66	Vert(LL)	-0.09	2-4	>872	240	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.32	Vert(TL)	-0.14	2-4	>590	180		
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2007/TPI2002		(Matrix)							Weight: 27 lb	FT = 20%

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD 2 X 4 SYP No.2	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 3=105/Mechanical, 2=333/0-5-11, 4=29/Mechanical
Max Horz 2=285(LC 5)
Max Uplift 3=-144(LC 5), 2=-402(LC 5)
Max Grav 3=105(LC 1), 2=333(LC 1), 4=97(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- NOTES** (10-13)
- 1) Wind: ASCE 7-05; 120mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60
 - 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - 4) All bearings are assumed to be SYP No.2 .
 - 5) Refer to girder(s) for truss to truss connections.
 - 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 144 lb uplift at joint 3 and 402 lb uplift at joint 2.
 - 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - 8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 40 lb up at 1-5-12, 40 lb up at 1-5-12, and 6 lb down and 27 lb up at 4-3-11, and 6 lb down and 27 lb up at 4-3-11 on top chord, and 16 lb up at 1-5-12, 16 lb up at 1-5-12, and 12 lb down at 4-3-11, and 12 lb down at 4-3-11 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
 - 9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
 - 10) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - 11) For special connections with reactions or uplifts less than 300 lbs. Use typical toe-nail connection (refer to BFS detail package)
 - 12) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

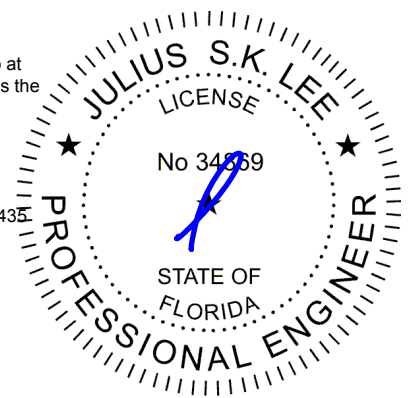
1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-3=-54, 2-4=-10

Concentrated Loads (lb)

Vert: 5=80(F=40, B=40) 6=53(F=27, B=27) 7=10(F=5, B=5) 8=-8(F=-4, B=-4)



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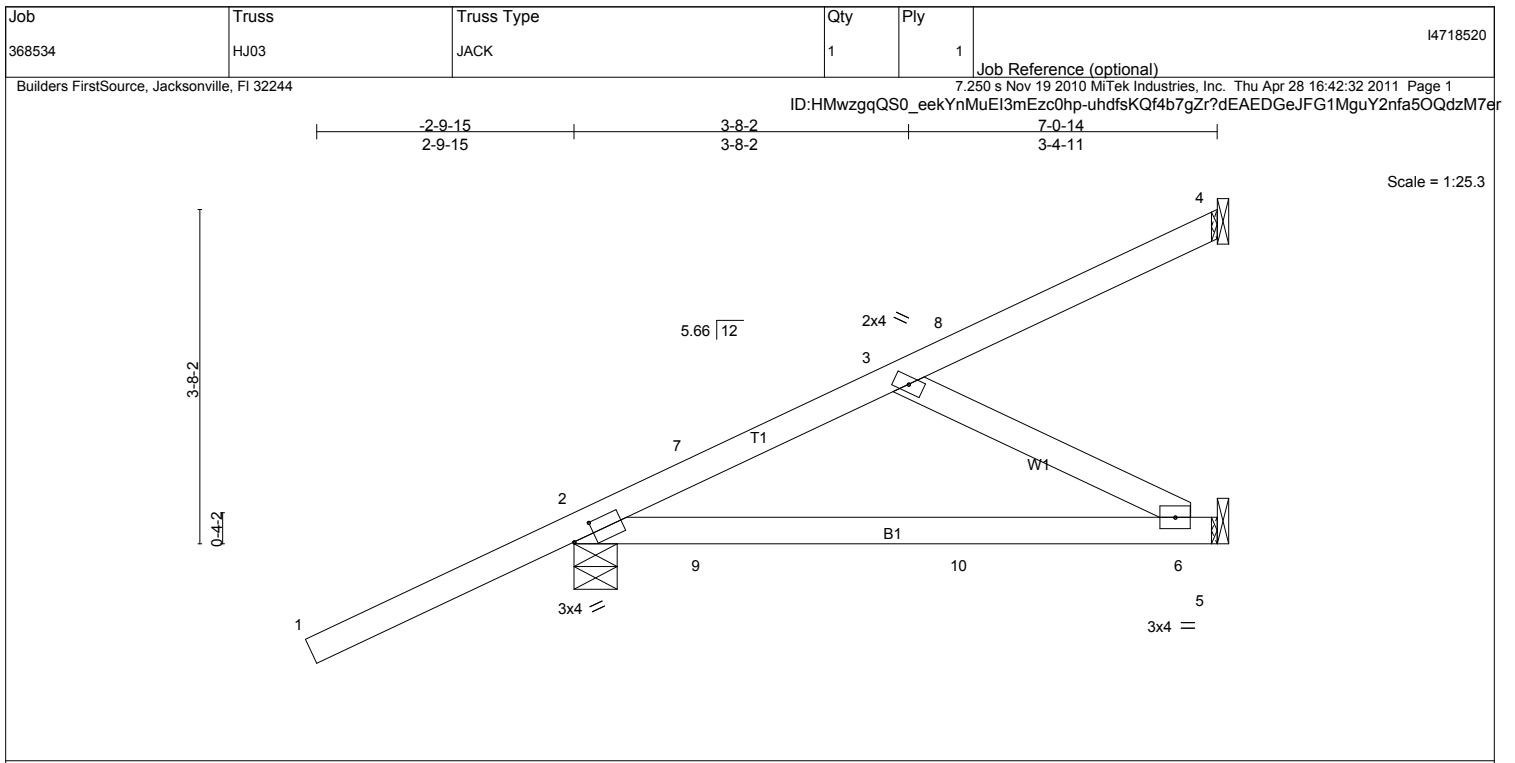
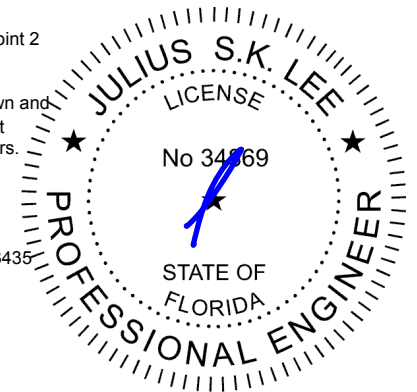


Plate Offsets (X,Y): [2:0-2-13,0-1-8]					
LOADING (psf)		SPACING	CSI	DEFL	PLATES GRIP
TCLL 20.0		2-0-0		in (loc) l/defl L/d	
TCDL 7.0		Plates Increase 1.25	TC 0.49	Vert(LL) -0.09 2-6 >866 240	MT20 244/190
BCLL 0.0 *		Lumber Increase 1.25	BC 0.38	Vert(TL) -0.16 2-6 >514 180	
BCDL 5.0		Rep Stress Incr NO	WB 0.15	Horz(TL) -0.01 5 n/a n/a	
		Code FBC2007/TPI2002	(Matrix)		Weight: 32 lb FT = 20%
LUMBER					
TOP CHORD 2 X 4 SYP No.1D					
BOT CHORD 2 X 4 SYP No.2					
WEBS 2 X 4 SYP No.3					
BRACING					
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.					
BOT CHORD Rigid ceiling directly applied or 8-6-8 oc bracing.					
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.					
REACTIONS (lb/size) 4=112/Mechanical, 2=584/0-5-11, 5=206/Mechanical					
Max Horz 2=285(LC 5)					
Max Uplift 4=-134(LC 5), 2=-682(LC 5), 5=-245(LC 6)					
Max Grav 4=112(LC 1), 2=584(LC 1), 5=219(LC 2)					
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.					
TOP CHORD 2-7=-491/581, 3-7=-372/466					
BOT CHORD 2-9=-533/363, 9-10=-533/363, 6-10=-533/363					
WEBS 3-6=-409/600					
NOTES (10-13)					
1) Wind: ASCE 7-05; 120mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60					
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.					
3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.					
4) All bearings are assumed to be SYP No.2.					
5) Refer to girder(s) for truss to truss connections.					
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 134 lb uplift at joint 4, 682 lb uplift at joint 2 and 245 lb uplift at joint 5.					
7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.					
8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 40 lb up at 1-5-12, 140 lb down and 243 lb up at 1-5-12, and 6 lb down and 27 lb up at 4-3-11, and 227 lb down and 344 lb up at 4-3-11 on top chord, and 16 lb up at 1-5-12, and 12 lb down at 4-3-11 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.					
9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).					
10) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.					
11) For special connections with reactions or uplifts less than 300 lbs. Use typical toe-nail connection (refer to BFS detail package)					
12) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435					
LOAD CASE(S) Standard					
1) Regular: Lumber Increase=1.25, Plate Increase=1.25					
Uniform Loads (plf)					
Vert: 1-4=-54, 2-5=-10					

Continued on page 2



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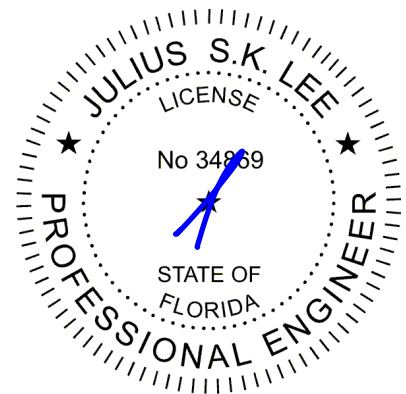
Job	Truss	Truss Type	Qty	Ply	
368534	HJ03	JACK	1	1	

I4718520

Job Reference (optional)

Builders FirstSource, Jacksonville, Fl 32244 7.250 s Nov 19 2010 MiTek Industries, Inc. Thu Apr 28 16:42:32 2011 Page 2
ID:HMwzgqQS0_eeKYNuEI3mEzc0hp-uhdfsKQf4b7gZr?dEAEDGeJFG1MguY2nfa5OQdzM7er

LOAD CASE(S) Standard
Concentrated Loads (lb)
Vert: 7=-100(F=40, B=-140) 8=-200(F=27, B=-227) 9=5(F) 10=-4(F)



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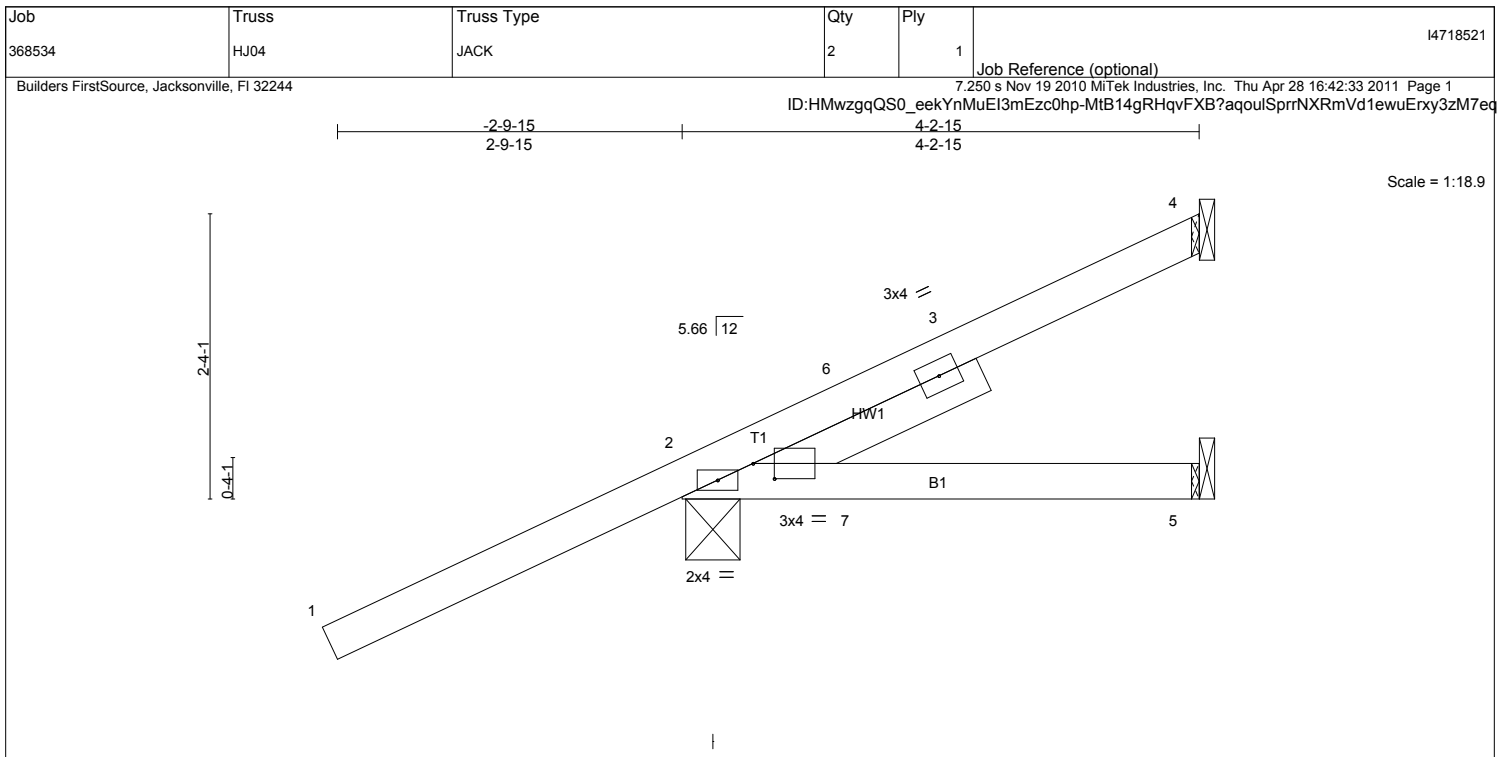
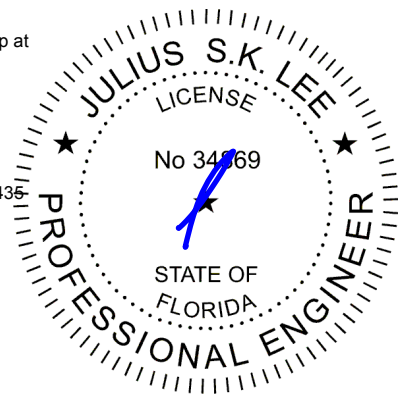


Plate Offsets (X,Y): [2:0-2-2,0-1-8]											
LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0		Plates Increase	1.25	TC 0.65		Vert(LL)	-0.01 2-5	>999	240	MT20	244/190
TCDL 7.0		Lumber Increase	1.25	BC 0.09		Vert(TL)	-0.01 2-5	>999	180		
BCLL 0.0 *		Rep Stress Incr	NO	WB 0.00		Horz(TL)	-0.00 4	n/a	n/a		
BCDL 5.0		Code FBC2007/TPI2002		(Matrix)						Weight: 21 lb	FT = 20%
LUMBER					BRACING						
TOP CHORD 2 X 4 SYP No.2					TOP CHORD Structural wood sheathing directly applied or 4-2-15 oc purlins.						
BOT CHORD 2 X 4 SYP No.2					BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.						
SLIDER Left 2 X 4 SYP No.2 2-0-5					<div>MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.</div>						
REACTIONS (lb/size) 4=15/Mechanical, 2=299/0-5-6, 5=16/Mechanical											
Max Horz 2=212(LC 5)											
Max Uplift 4=-60(LC 6), 2=-403(LC 5)											
Max Grav 4=15(LC 1), 2=299(LC 1), 5=49(LC 2)											
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.											
NOTES (10-13)											
1) Wind: ASCE 7-05; 120mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60											
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.											
3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.											
4) All bearings are assumed to be SYP No.2 .											
5) Refer to girder(s) for truss to truss connections.											
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 60 lb uplift at joint 4 and 403 lb uplift at joint 2.											
7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.											
8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 40 lb up at 1-5-12, and 40 lb up at 1-5-12 on top chord, and 16 lb up at 1-5-12, and 16 lb up at 1-5-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.											
9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).											
10) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.											
11) For special connections with reactions or uplifts less than 300 lbs. Use typical toe-nail connection (refer to BFS detail package)											
12) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435											
LOAD CASE(S) Standard											
1) Regular: Lumber Increase=1.25, Plate Increase=1.25											
Uniform Loads (plf)											
Vert: 1-4=-54, 2-5=-10											
Concentrated Loads (lb)											
Vert: 6=80(F=40, B=40) 7=10(F=5, B=5)											

April 28,2011



April 28, 2011



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

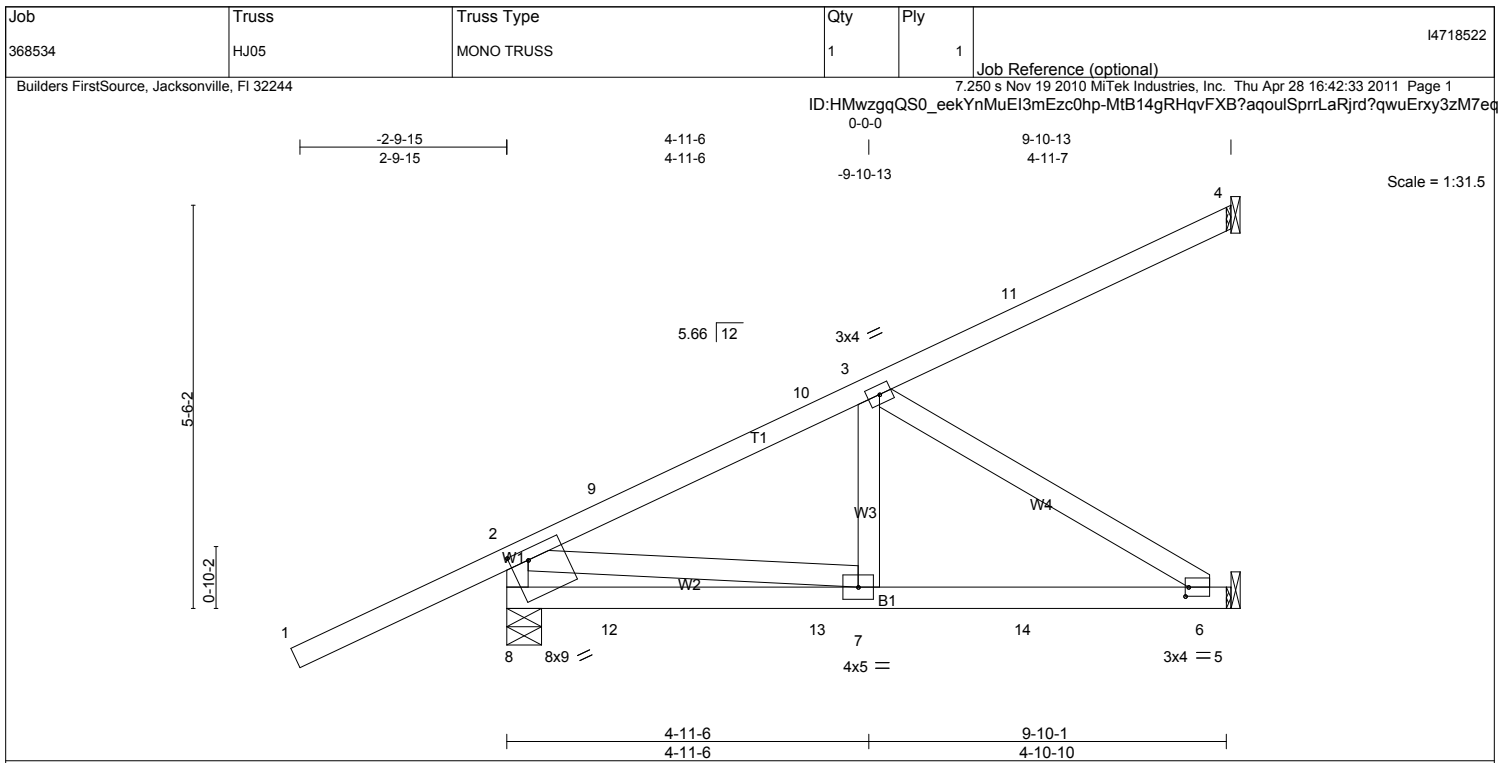


Plate Offsets (X,Y): [6:0-0-8,0-1-8], [8:0-3-0,0-1-12]							
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d
TCLL	20.0	Plates Increase	1.25	TC	0.78	Vert(LL)	-0.04
TCDL	7.0	Lumber Increase	1.25	BC	0.32	Vert(TL)	-0.08
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.18	Horz(TL)	-0.01
BCDL	5.0	Code FBC2007/TPI2002	(Matrix)				
				PLATES		GRIP	
				MT20		244/190	
				Weight: 55 lb		FT = 20%	

LUMBER		BRACING	
TOP CHORD 2 X 4 SYP No.2		TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.	
BOT CHORD 2 X 4 SYP No.2		BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.	
WEBS 2 X 4 SYP No.3		MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.	

REACTIONS (lb/size) 8=413/0-5-11, 4=145/Mechanical, 5=194/Mechanical
Max Horz 8=361(LC 5)
Max Uplift 8=-494(LC 5), 4=-178(LC 5), 5=-229(LC 5)
Max Grav 8=413(LC 1), 4=145(LC 1), 5=226(LC 2)

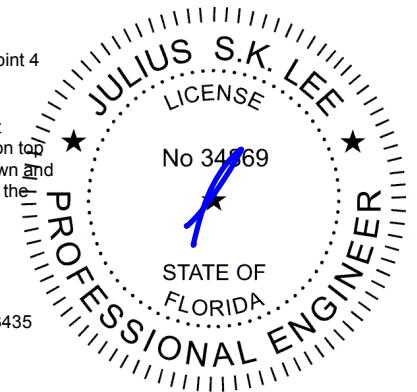
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-8=-421/496, 2-9=-330/292, 9-10=-280/282, 3-10=-276/251
BOT CHORD 8-12=-280/52, 12-13=-280/52, 7-13=-280/52, 7-14=-402/277, 6-14=-402/277
WEBS 2-7=-420/562, 3-6=-326/472

- NOTES** (10-13)
- 1) Wind: ASCE 7-05; 120mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone; Lumber DOL=1.60 plate grip DOL=1.60
 - 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - 4) All bearings are assumed to be SYP No.2.
 - 5) Refer to girder(s) for truss to truss connections.
 - 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 494 lb uplift at joint 8, 178 lb uplift at joint 4 and 229 lb uplift at joint 5.
 - 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - 8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 39 lb up at 1-5-12, 39 lb up at 1-5-12, 42 lb up at 4-3-11, 42 lb up at 4-3-11, and 48 lb down and 123 lb up at 7-1-10, and 48 lb down and 123 lb up at 7-1-10 on top chord, and 28 lb up at 1-5-12, 28 lb up at 1-5-12, 5 lb down and 9 lb up at 4-3-11, 5 lb down and 9 lb up at 4-3-11, and 36 lb down and 10 lb up at 7-1-10, and 36 lb down and 10 lb up at 7-1-10 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
 - 9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
 - 10) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - 11) For special connections with reactions or uplifts less than 300 lbs. Use typical toe-nail connection (refer to BFS detail package)
 - 12) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

Continued on page 2

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Job	Truss	Truss Type	Qty	Ply	
368534	HJ05	MONO TRUSS	1	1	

I4718522

Job Reference (optional)

Builders FirstSource, Jacksonville, FL 32244

7.250 s Nov 19 2010 MiTek Industries, Inc. Thu Apr 28 16:42:33 2011 Page 2

ID:HMwzggQS0_eeKYNmUEl3mEzc0hp-MtB14gRHqvFXB?aqouLSprLaRjrd?quwErxy3zM7eq

LOAD CASE(S) Standard

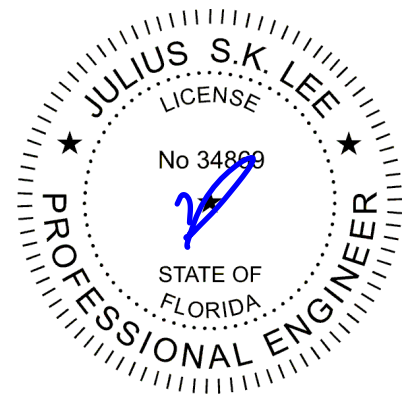
1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-2=-54, 2-4=-54, 5-8=-10

Concentrated Loads (lb)

Vert: 9=78(F=39, B=39) 10=27(F=14, B=14) 11=-95(F=-48, B=-48) 12=33(F=16, B=16) 13=18(F=9, B=9) 14=-27(F=-14, B=-14)



April 28, 2011



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

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Boynton, FL 33435

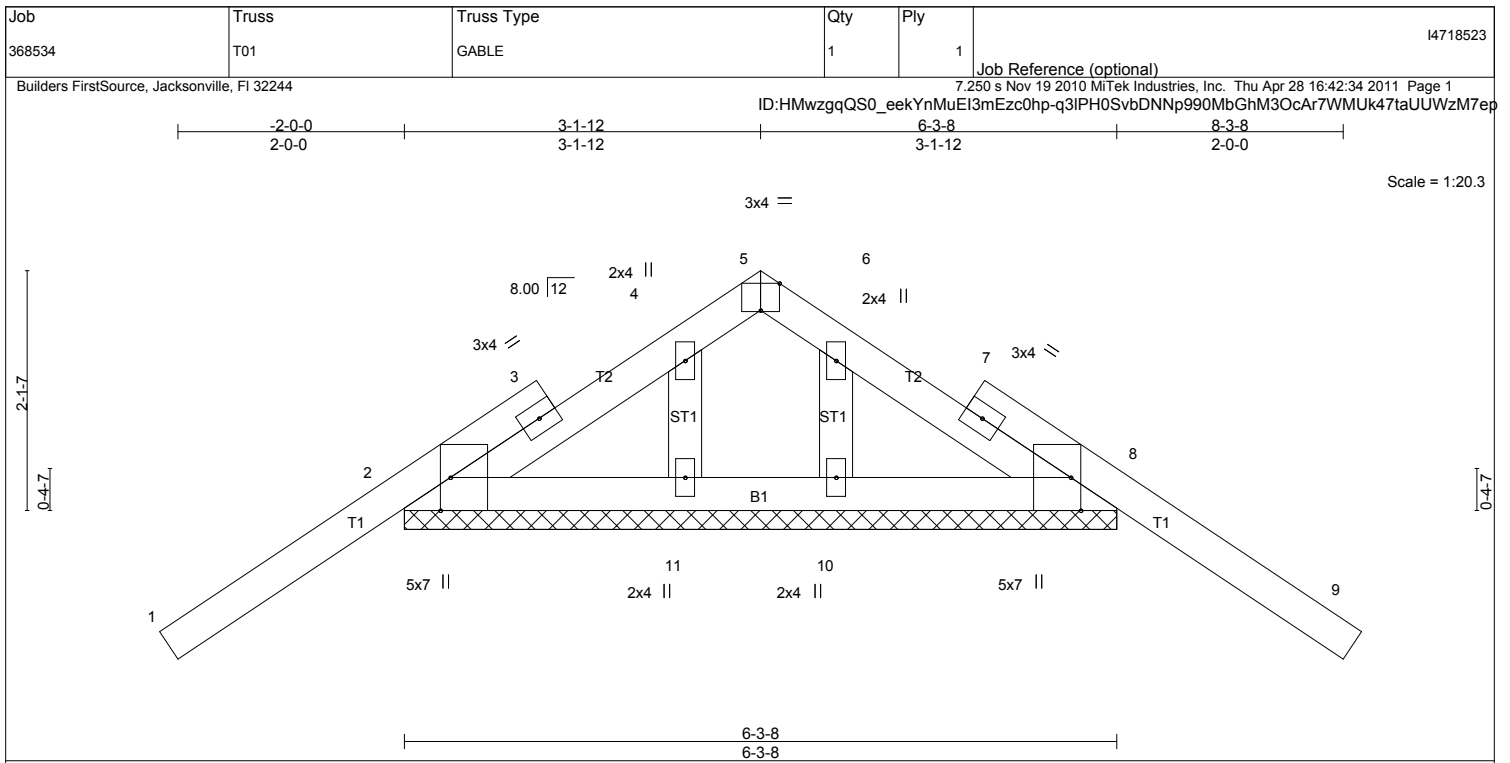


Plate Offsets (X,Y): [2:0-3-8,Edge], [5:0-2-0,Edge], [8:0-3-8,Edge]					
LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.40	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.04	Vert(LL) -0.03 9 n/r 120		
BCLL 0.0 *	Rep Stress Incr NO	WB 0.01	Vert(TL) -0.04 9 n/r 90		
BCDL 5.0	Code FBC2007/TPI2002	(Matrix)	Horz(TL) 0.00 8 n/a n/a		
				Weight: 34 lb	FT = 20%

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD 2 X 4 SYP No.2	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
OTHERS 2 X 4 SYP No.3	

REACTIONS All bearings 6-3-8.
 (lb) - Max Horz 2=92(LC 6)
 Max Uplift All uplift 100 lb or less at joint(s) 11, 10 except 2=287(LC 6), 8=291(LC 7)
 Max Grav All reactions 250 lb or less at joint(s) 11, 10 except 2=250(LC 1), 8=250(LC 1)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- NOTES** (11-14)
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-05; 120mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002.
 - Gable requires continuous bottom chord bearing.
 - Gable studs spaced at 1-4-0 oc.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - All bearings are assumed to be SYP No.2.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 11, 10 except (jt=lb) 2=287, 8=291.
 - "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - For special connections with reactions or uplifts less than 300 lbs. Use typical toe-nail connection (refer to BFS detail package)
 - Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

April 28, 2011

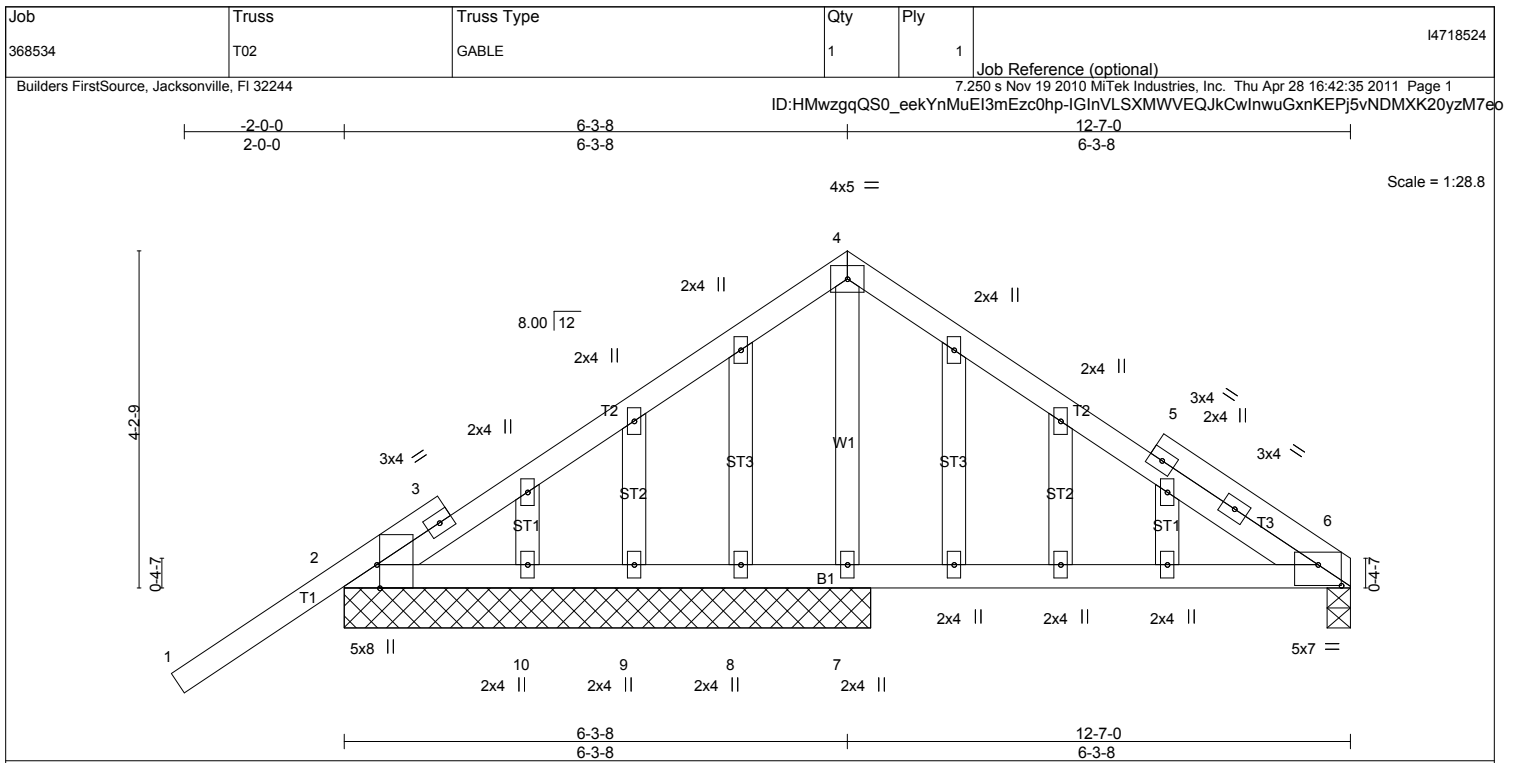


Plate Offsets (X,Y): [2:0-3-8,Edge], [6:0-3-8,0-3-2]					
LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.38	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.23	Vert(LL) -0.03 6-7 >999 240		
BCLL 0.0 *	Lumber Increase 1.25	WB 0.18	Vert(TL) -0.06 6-7 >999 180		
BCDL 5.0	Rep Stress Incr NO	(Matrix)	Horz(TL) 0.01 6 n/a n/a		
	Code FBC2007/TPI2002			Weight: 72 lb	FT = 20%

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 10-0-0 oc purlins.
BOT CHORD 2 X 4 SYP No.2	BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 2 X 4 SYP No.3	
OTHERS 2 X 4 SYP No.3	

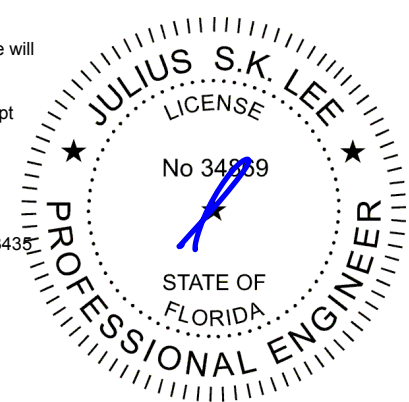
REACTIONS All bearings 6-7-0 except (jt=length) 6=0-3-8.
 (lb) - Max Horz 2=198(LC 5)
 Max Uplift All uplift 100 lb or less at joint(s) 6, 8, 9, 10 except 2=-142(LC 6), 7=-504(LC 7)
 Max Grav All reactions 250 lb or less at joint(s) 2, 6, 8, 9, 10 except 7=716(LC 1)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-3=-277/199, 3-4=-242/348, 4-5=-274/361, 5-6=-290/251
 BOT CHORD 2-10=-223/334, 9-10=-223/334, 8-9=-223/334, 7-8=-223/334, 6-7=-223/334
 WEBS 4-7=-609/534

- NOTES** (10-13)
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-05; 120mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002.
 - Gable studs spaced at 1-4-0 oc.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - All bearings are assumed to be SYP No.2.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 6, 8, 9, 10 except (jt=lb) 2=142, 7=504.
 - "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - For special connections with reactions or uplifts less than 300 lbs. Use typical toe-nail connection (refer to BFS detail package)
 - Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

April 28, 2011



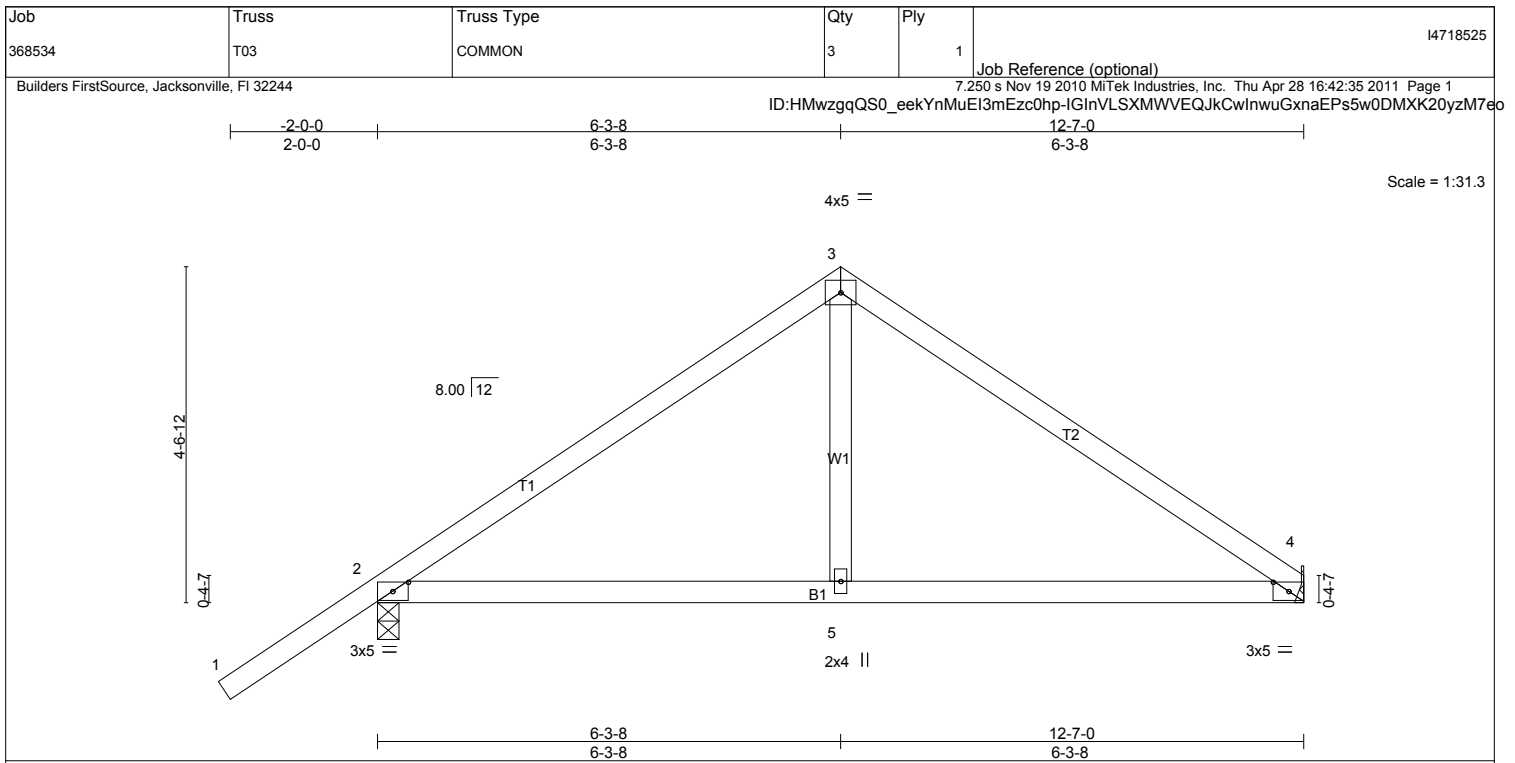


Plate Offsets (X,Y): [2:0-2-9,0-1-8], [4:0-2-9,0-1-8]									
LOADING (psf)	SPACING		CSI	DEFL	in	(loc)	I/defl	L/d	PLATES GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.36	Vert(LL) 0.04	4-5	>999	240	MT20	244/190	
TCDL 7.0	Lumber Increase 1.25	BC 0.22	Vert(TL) -0.07	4-5	>999	180			
BCLL 0.0 *	Rep Stress Incr YES	WB 0.07	Horz(TL) 0.01	4	n/a	n/a			
BCDL 5.0	Code FBC2007/TPI2002	(Matrix)							
					Weight: 51 lb FT = 20%				

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD 2 X 4 SYP No.2	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 2 X 4 SYP No.3	

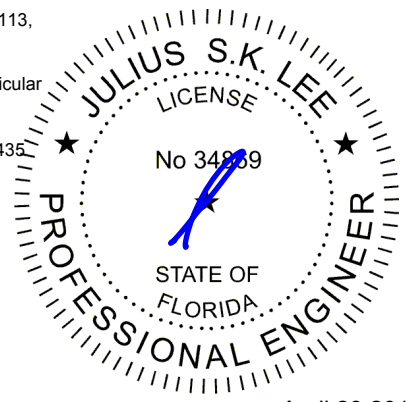
REACTIONS (lb/size) 4=385/Mechanical, 2=527/0-3-8
Max Horz 2=170(LC 5)
Max Uplift 4=113(LC 7), 2=-245(LC 6)

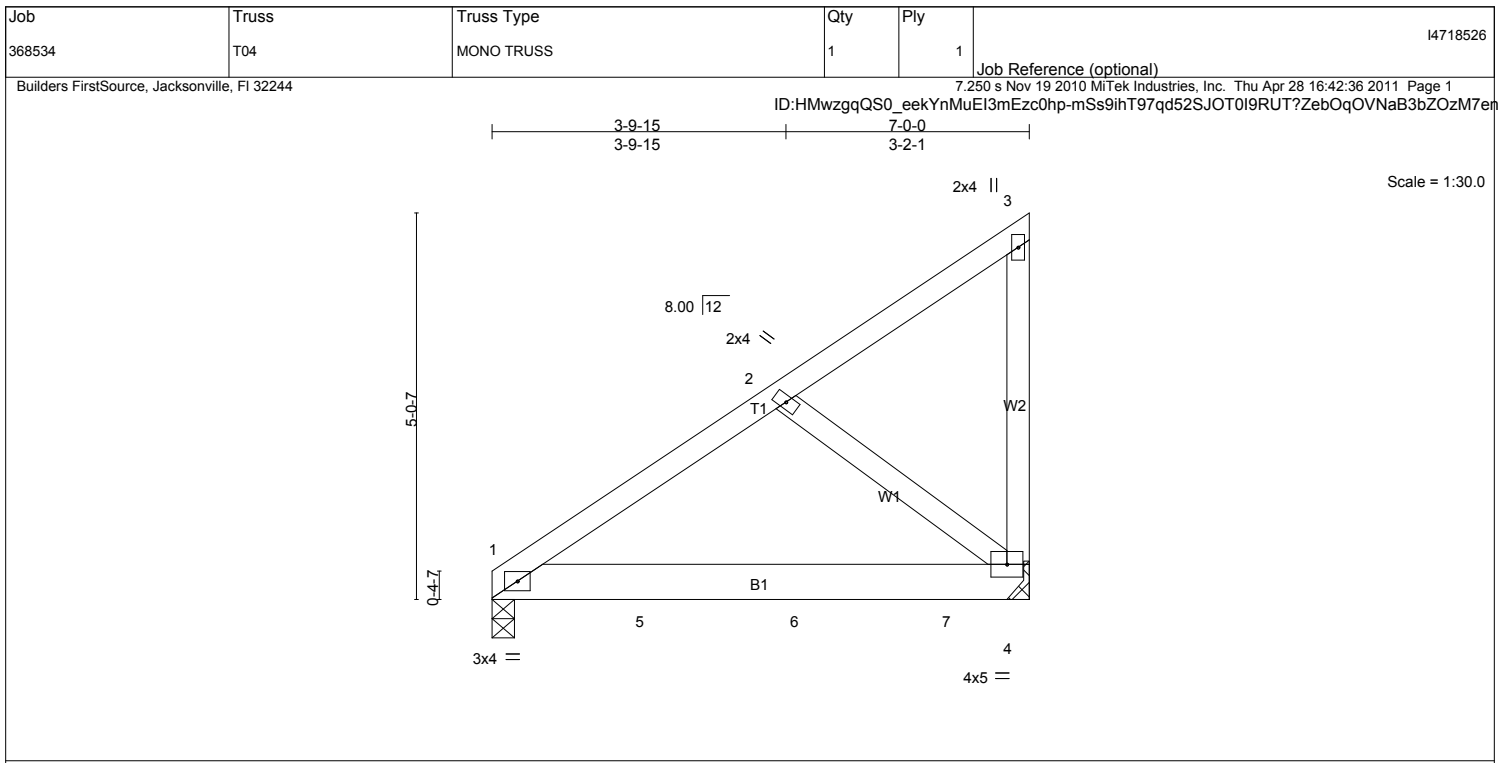
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-481/248, 3-4=-471/231
BOT CHORD 2-5=-67/315, 4-5=-67/315

- NOTES** (9-13)
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-05; 120mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - All bearings are assumed to be SYP No.2 .
 - Refer to girder(s) for truss to truss connections.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 4=113, 2=245.
 - "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - For special connections with reactions or uplifts less than 300 lbs. Use typical toe-nail connection (refer to BFS detail package)
 - Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435
 - Use Simpson HTU26 to attach Truss to Carrying member

LOAD CASE(S) Standard

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LOADING (psf)	SPACING	CSI	DEFL	in (loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.16	Vert(LL)	-0.14	1-4	>559	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.84	Vert(TL)	-0.28	1-4	>283		
BCLL 0.0 *	Lumber Increase 1.25	WB 0.06	Horz(TL)	0.00	4	n/a		
BCDL 5.0	Rep Stress Incr NO	(Matrix)						
	Code FBC2007/TPI2002						Weight: 41 lb	FT = 20%

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD 2 X 6 SYP No.1D	BOT CHORD Rigid ceiling directly applied or 6-10-11 oc bracing.
WEBS 2 X 4 SYP No.3	

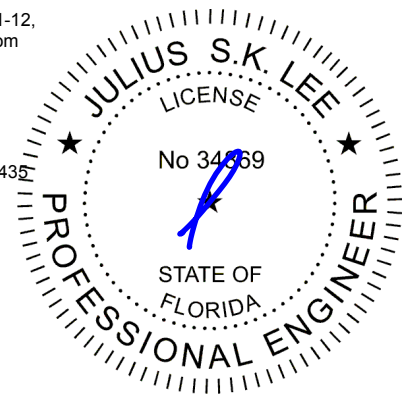
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 1=777/0-3-8, 4=872/Mechanical
Max Horz 1=173(LC 5)
Max Uplift 1=230(LC 5), 4=-362(LC 5)

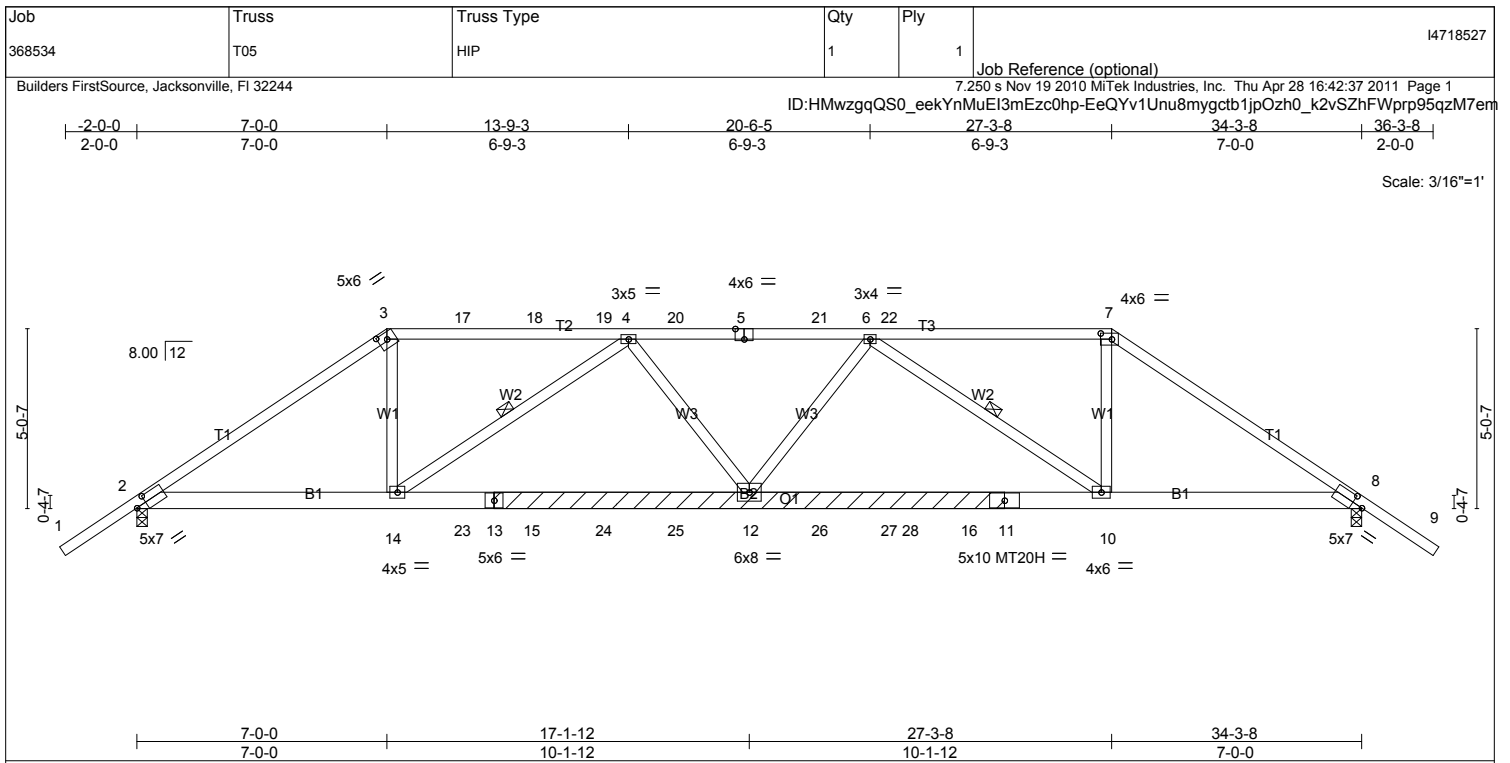
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- NOTES** (10-14)
- 1) Wind: ASCE 7-05; 120mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60
 - 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - 4) All bearings are assumed to be SYP No.2 .
 - 5) Refer to girder(s) for truss to truss connections.
 - 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=230, 4=362.
 - 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - 8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 94 lb down and 49 lb up at 0-1-12, 375 lb down and 127 lb up at 2-0-12, and 375 lb down and 127 lb up at 4-0-12, and 375 lb down and 127 lb up at 6-0-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
 - 9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
 - 10) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - 11) For special connections with reactions or uplifts less than 300 lbs. Use typical toe-nail connection (refer to BFS detail package)
 - 12) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435
 - 13) Use Simpson HTU26 to attach Truss to Carrying member

- LOAD CASE(S)** Standard
- 1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-4=-10, 1-3=-54
Concentrated Loads (lb)
Vert: 1=-94(B) 5=-375(B) 6=-375(B) 7=-375(B)



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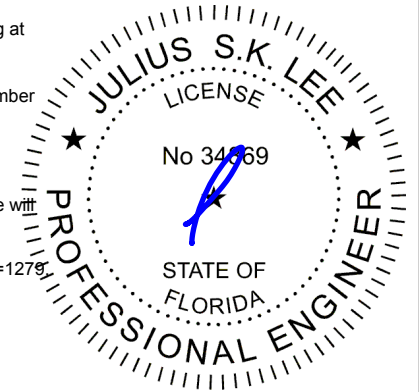
LOADING (psf)		SPACING		CSI		DEFL		PLATES		GRIP	
TCLL	20.0	Plates Increase	1.25	TC	0.96	Vert(LL)	0.43 10-12 >958 240	MT20	244/190		
TCDL	7.0	Lumber Increase	1.25	BC	0.98	Vert(TL)	-0.73 10-12 >556 180	MT20H	187/143		
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.67	Horz(TL)	0.13 8 n/a n/a				
BCDL	5.0	Code FBC2007/TPI2002		(Matrix)						Weight: 228 lb	FT = 20%

LUMBER		BRACING	
TOP CHORD	2 X 4 SYP No.2 *Except*	TOP CHORD	Structural wood sheathing directly applied.
	T2: 2 X 4 SYP No.1D	BOT CHORD	Rigid ceiling directly applied or 5-3-5 oc bracing.
BOT CHORD	2 X 6 SYP No.1D	WEBS	1 Row at midpt 4-14, 6-10
WEBS	2 X 4 SYP No.3		
OTHERS	2 X 6 SYP No.1D		
LBR SCAB	11-13 2 X 6 SYP No.1D one side		

REACTIONS (lb/size)	
2=2374/0-3-8, 8=2246/0-3-8	
Max Horz 2=-151(LC 3)	
Max Uplift 2=-1279(LC 5), 8=-1020(LC 6)	

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.	
TOP CHORD	2-3=-3716/2010, 3-17=-3031/1742, 17-18=-3030/1742, 18-19=-3030/1742, 4-19=-3030/1742, 4-20=-4863/2457, 5-20=-4863/2457, 5-21=-4863/2457, 6-21=-4863/2457, 6-22=-2977/1491, 7-22=-2978/1491, 7-8=-3652/1707
BOT CHORD	2-14=-1696/2987, 14-23=-2492/4626, 13-23=-2492/4626, 13-15=-2492/4626, 15-24=-2492/4626, 24-25=-2492/4626, 12-25=-2492/4626, 12-26=-2286/4528, 26-27=-2286/4528, 27-28=-2286/4528, 16-28=-2286/4528, 11-16=-2286/4528, 10-11=-2286/4528, 8-10=-1288/2926
WEBS	3-14=-639/1392, 4-14=-2015/1097, 4-12=0/580, 6-12=-243/695, 6-10=-1958/1209, 7-10=-817/1678

- NOTES (13-16)**
- Attached 14-3-8 scab 11 to 13, back face(s) 2 X 6 SYP No.1D with 2 row(s) of 10d (0.131"x3") nails spaced 9" o.c. except : starting at 0-0-0 from end at joint 11, nail 2 row(s) at 4" o.c. for 8-1-12.
 - Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-05; 120mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60
 - Provide adequate drainage to prevent water ponding.
 - All plates are MT20 plates unless otherwise indicated.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - All bearings are assumed to be SYP No.2.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=1279, 8=1020.
 - "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.



Job	Truss	Truss Type	Qty	Ply	
368534	T05	HIP	1	1	

I4718527

Builders FirstSource, Jacksonville, FL 32244

Job Reference (optional)

7.250 s Nov 19 2010 MiTek Industries, Inc. Thu Apr 28 16:42:37 2011 Page 2

ID:HMwzqgQS0_eeKYnMuEI3mEzc0hp-EeQYv1Unu8mygctb1jpOzh0_k2vSZhFWprp95qzM7em

NOTES (13-16)

- 11) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 201 lb down and 284 lb up at 7-0-0, 110 lb down and 120 lb up at 9-0-12, 110 lb down and 120 lb up at 11-0-12, 110 lb down and 120 lb up at 13-0-12, 110 lb down and 120 lb up at 15-0-12, 110 lb down and 120 lb up at 17-0-12, and 110 lb down and 120 lb up at 19-0-12, and 110 lb down and 120 lb up at 21-0-12 on top chord, and 272 lb down and 165 lb up at 7-0-0, 72 lb down at 9-0-12, 72 lb down at 11-0-12, 72 lb down at 13-0-12, 72 lb down at 15-0-12, 72 lb down at 17-0-12, 72 lb down at 19-0-12, and 72 lb down at 21-0-12, and 862 lb down and 374 lb up at 21-7-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 12) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- 13) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 14) For special connections with reactions or uplifts less than 300 lbs. Use typical toe-nail connection (refer to BFS detail package)
- 15) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

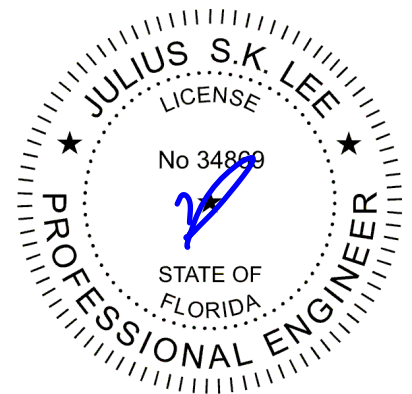
- 1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-3=-54, 3-7=-54, 7-9=-54, 2-8=-10

Concentrated Loads (lb)

Vert: 3=-201(B) 5=-110(B) 14=-205(B) 12=-24(B) 15=-24(B) 17=-110(B) 18=-110(B) 19=-110(B) 20=-110(B) 21=-110(B) 22=-110(B) 23=-24(B) 24=-24(B) 25=-24(B) 26=-24(B) 27=-24(B) 28=-862(B)

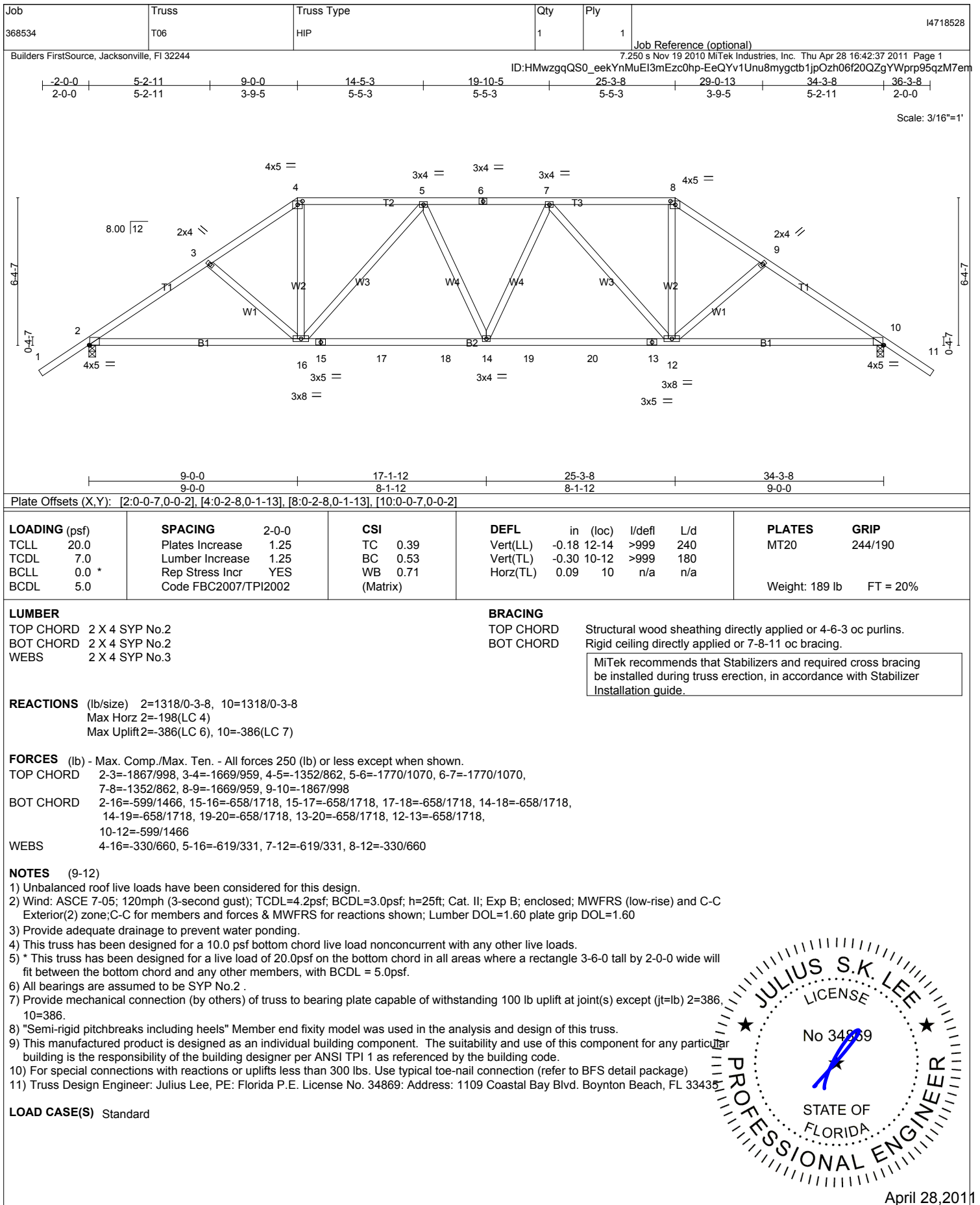


April 28, 2011

**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.**

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

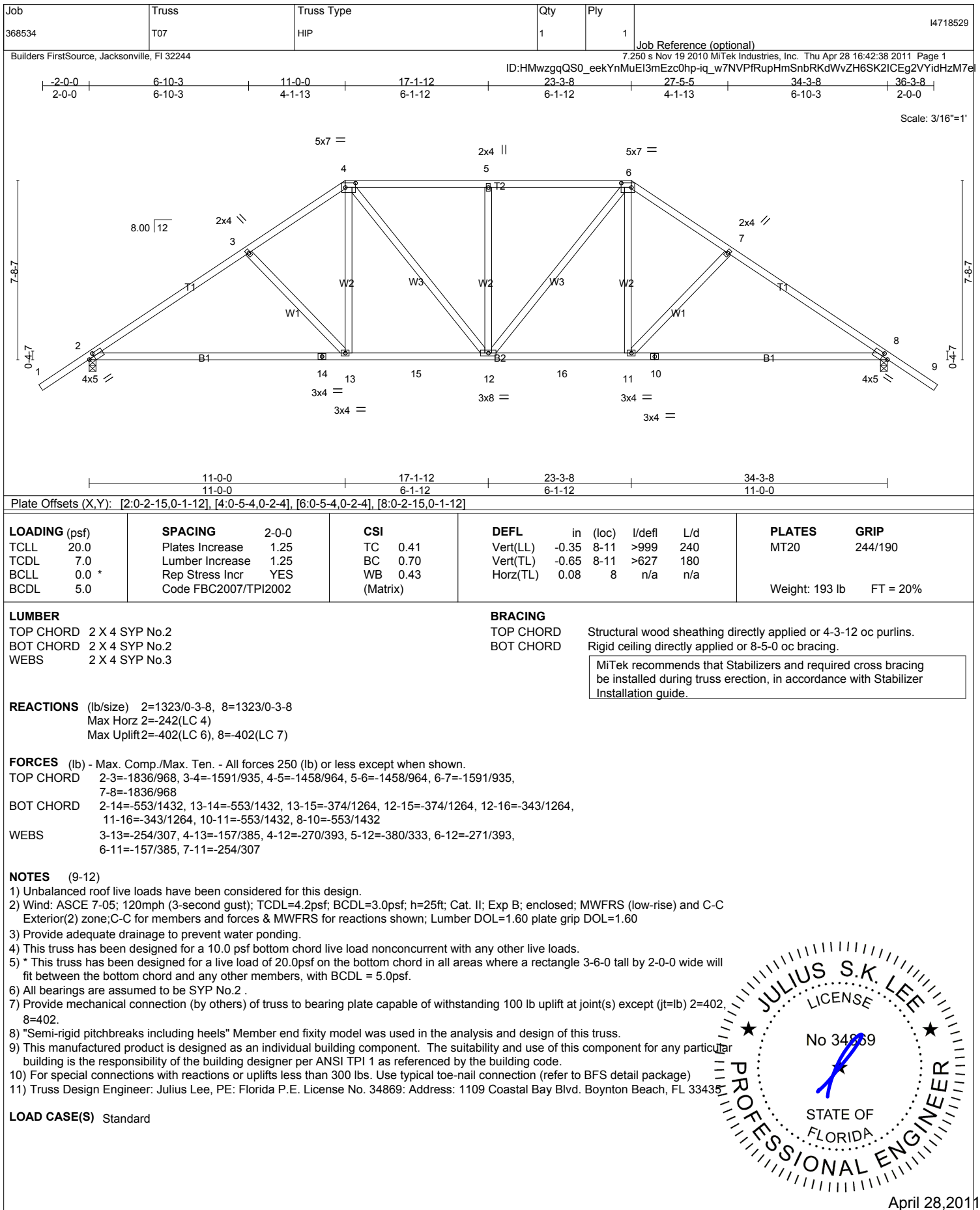
Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435



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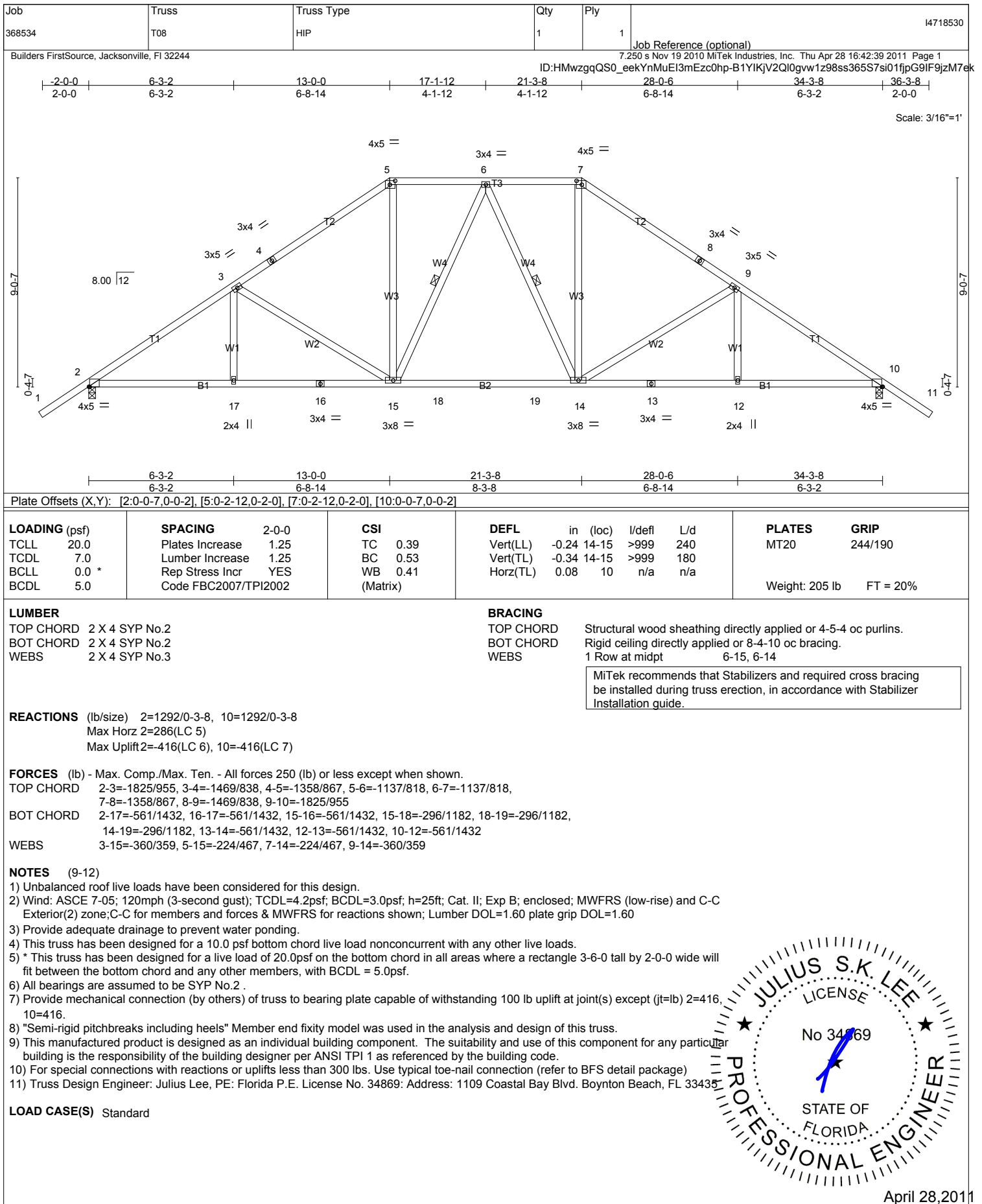
Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435



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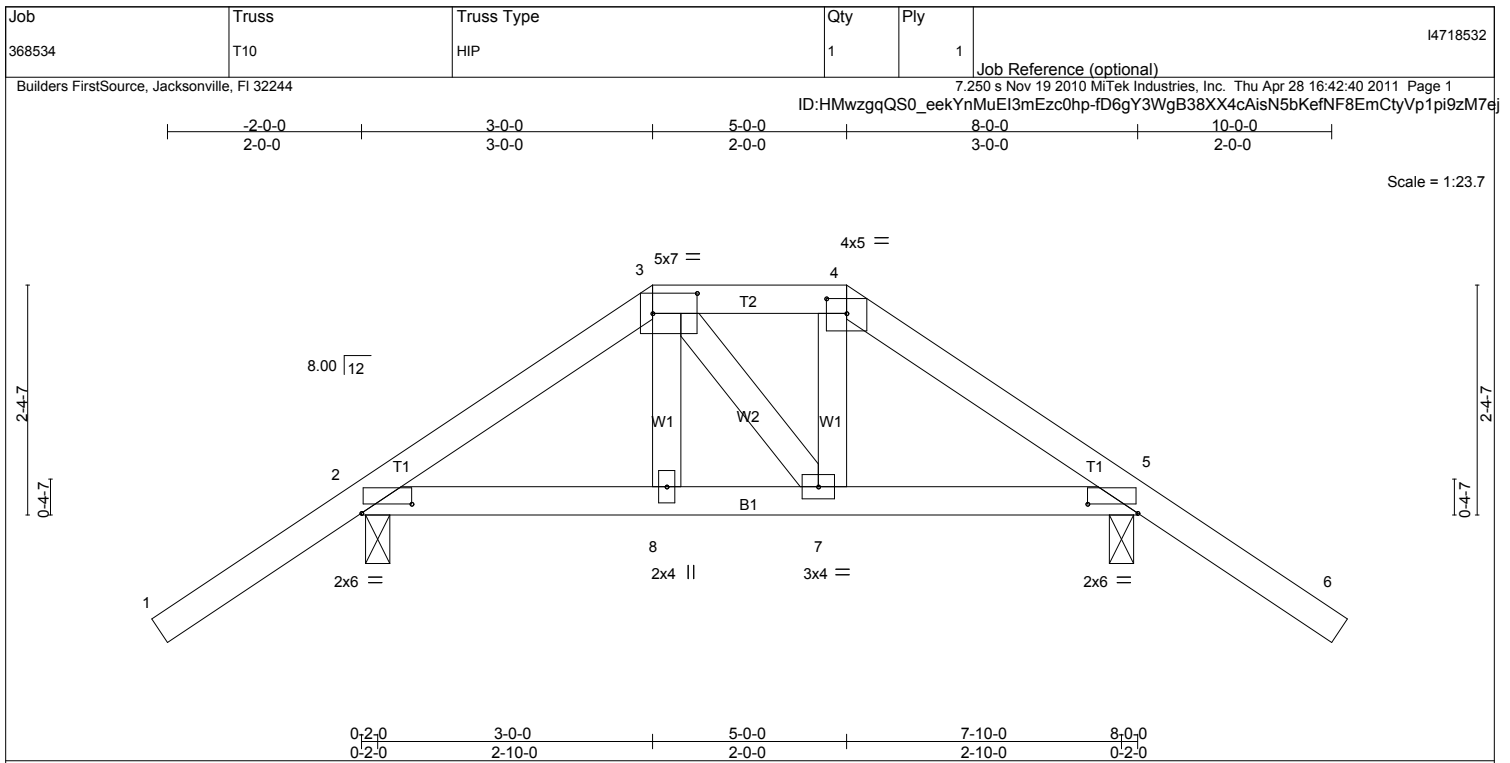


Plate Offsets (X,Y): [2:0-6-3,0-1-2], [3:0-5-8,0-2-8], [4:0-2-8,0-1-13], [5:0-6-3,0-1-2]															
LOADING (psf)		SPACING		2-0-0		CSI		DEFL		in (loc) l/defl L/d		PLATES		GRIP	
TCLL 20.0		Plates Increase		1.25		TC 0.29		Vert(LL)		-0.00 2-8 >999 240		MT20		244/190	
TCDL 7.0		Lumber Increase		1.25		BC 0.14		Vert(TL)		-0.01 2-8 >999 180					
BCLL 0.0 *		Rep Stress Incr		NO		WB 0.03		Horz(TL)		0.00 5 n/a n/a					
BCDL 5.0		Code FBC2007/TPI2002				(Matrix)						Weight: 42 lb		FT = 20%	

LUMBER		BRACING	
TOP CHORD 2 X 4 SYP No.2		TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD 2 X 4 SYP No.2		BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 2 X 4 SYP No.3			

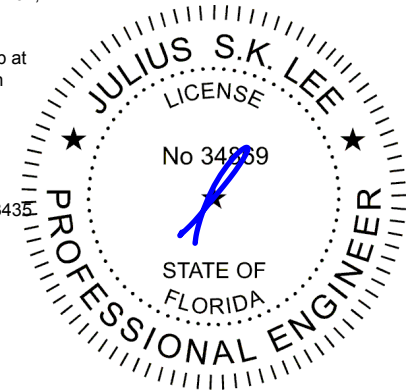
REACTIONS (lb/size) 2=312/0-3-0, 5=312/0-3-0
Max Horz 2=88(LC 5)
Max Uplift 2=-262(LC 5), 5=-253(LC 6)
Max Grav 2=319(LC 9), 5=312(LC 1)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- NOTES** (11-14)
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-05; 120mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - All bearings are assumed to be SYP No.2 .
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=262, 5=253.
 - "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 68 lb up at 3-0-0, and 73 lb up at 5-0-0 on top chord, and 31 lb down at 3-0-0, and 31 lb down at 4-11-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
 - In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
 - This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - For special connections with reactions or uplifts less than 300 lbs. Use typical toe-nail connection (refer to BFS detail package)
 - Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-3=-54, 3-4=-54, 4-6=-54, 2-5=-10
Concentrated Loads (lb)
Vert: 3=65(B) 4=65(B) 8=-10(B) 7=-10(B)



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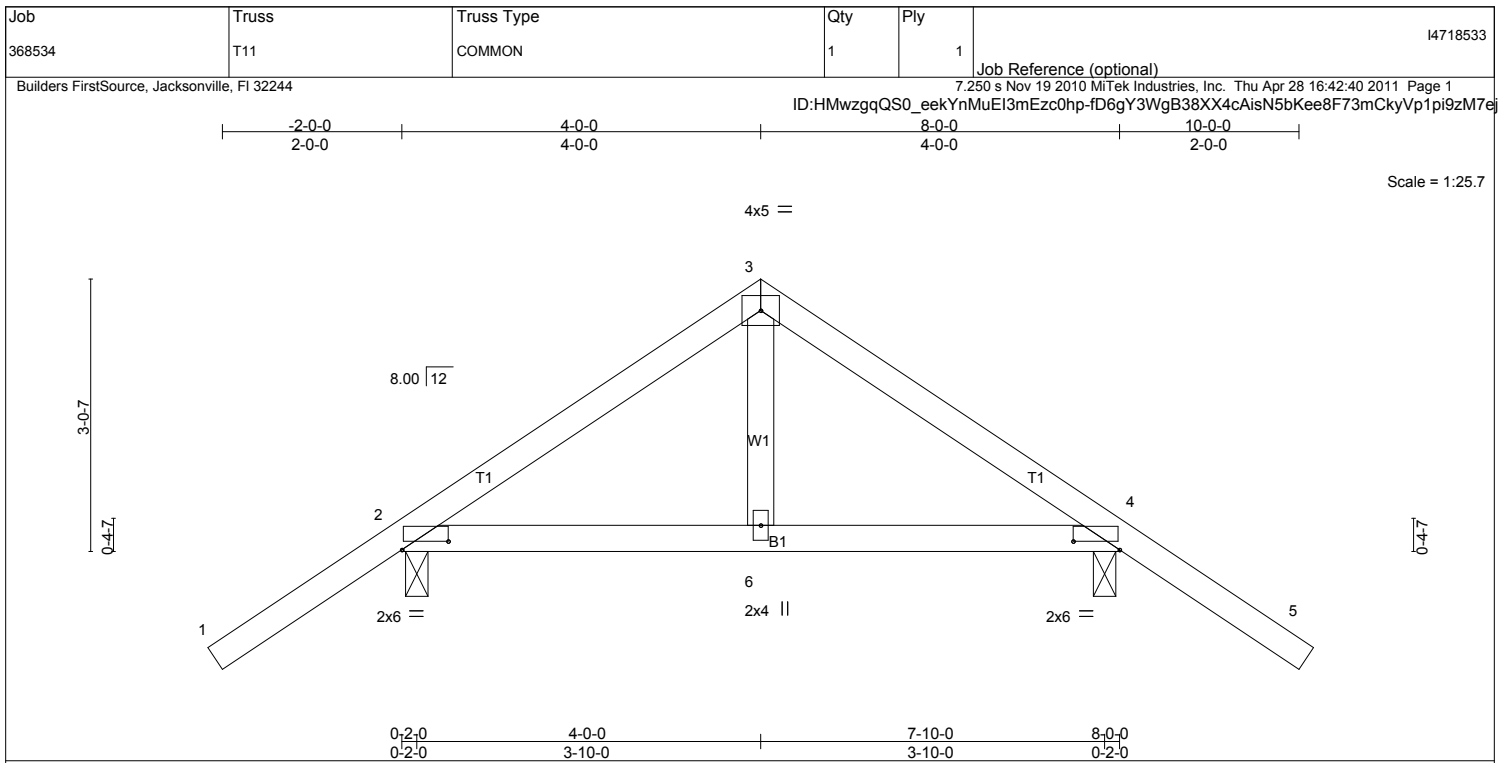


Plate Offsets (X,Y): [2:0-6-3,0-1-2], [4:0-6-3,0-1-2]							
LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	I/defl	L/d
TCLL 20.0	Plates Increase 1.25	TC 0.37	Vert(LL)	-0.01	4-6	>999	240
TCDL 7.0	Lumber Increase 1.25	BC 0.22	Vert(TL)	-0.01	4-6	>999	180
BCCL 0.0 *	Rep Stress Incr YES	WB 0.04	Horz(TL)	0.00	4	n/a	n/a
BCDL 5.0	Code FBC2007/TPI2002	(Matrix)					
				Weight: 37 lb FT = 20%			

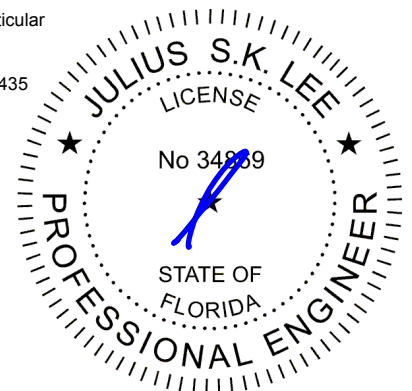
LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 6'-0" oc purlins.
BOT CHORD 2 X 4 SYP No.2	BOT CHORD Rigid ceiling directly applied or 10'-0" oc bracing.
WEBS 2 X 4 SYP No.3	

REACTIONS (lb/size) 2=367/0-3-0, 4=367/0-3-0
Max Horz 2=97(LC 6)
Max Uplift 2=-205(LC 6), 4=-205(LC 7)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- NOTES** (8-11)
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-05; 120mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3'-6" tall by 2'-0" wide will fit between the bottom chord and any other members.
 - All bearings are assumed to be SYP No.2.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=205, 4=205.
 - "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - For special connections with reactions or uplifts less than 300 lbs. Use typical toe-nail connection (refer to BFS detail package)
 - Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



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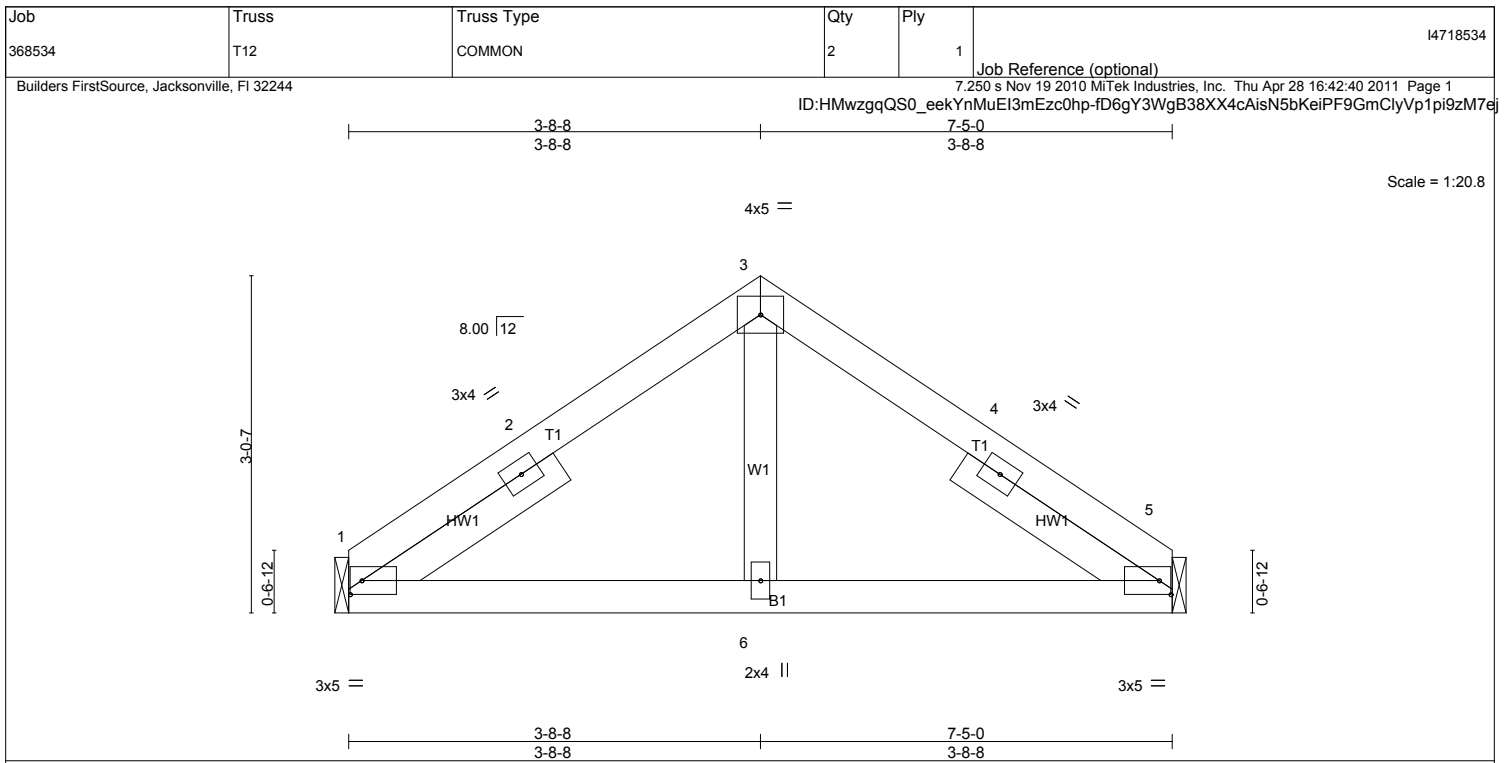


Plate Offsets (X,Y): [1:0-1-4,0-1-8], [5:0-1-4,0-1-8]											
LOADING (psf)		SPACING 2-0-0		CSI		DEFL in (loc) l/defl L/d			PLATES GRIP		
TCLL	20.0	Plates Increase 1.25		TC	0.10	Vert(LL)	-0.00	5-6	>999	240	MT20 244/190
TCDL	7.0	Lumber Increase 1.25		BC	0.08	Vert(TL)	-0.01	5-6	>999	180	
BCLL	0.0 *	Rep Stress Incr YES		WB	0.04	Horz(TL)	0.00	5	n/a	n/a	
BCDL	5.0	Code FBC2007/TPI2002		(Matrix)							Weight: 35 lb FT = 20%

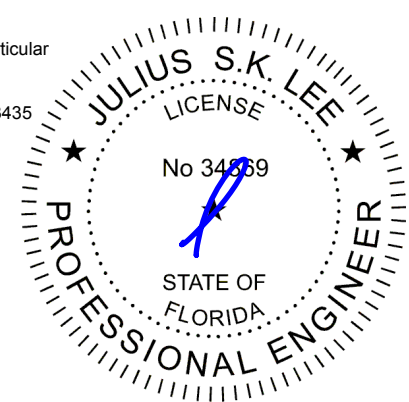
LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD 2 X 4 SYP No.2	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 2 X 4 SYP No.3	
SLIDER Left 2 X 4 SYP No.2 2-0-14, Right 2 X 4 SYP No.2 2-0-14	

REACTIONS (lb/size) 1=233/Mechanical, 5=233/Mechanical
Max Horz 1=90(LC 5)
Max Uplift 1=-67(LC 6), 5=-67(LC 7)

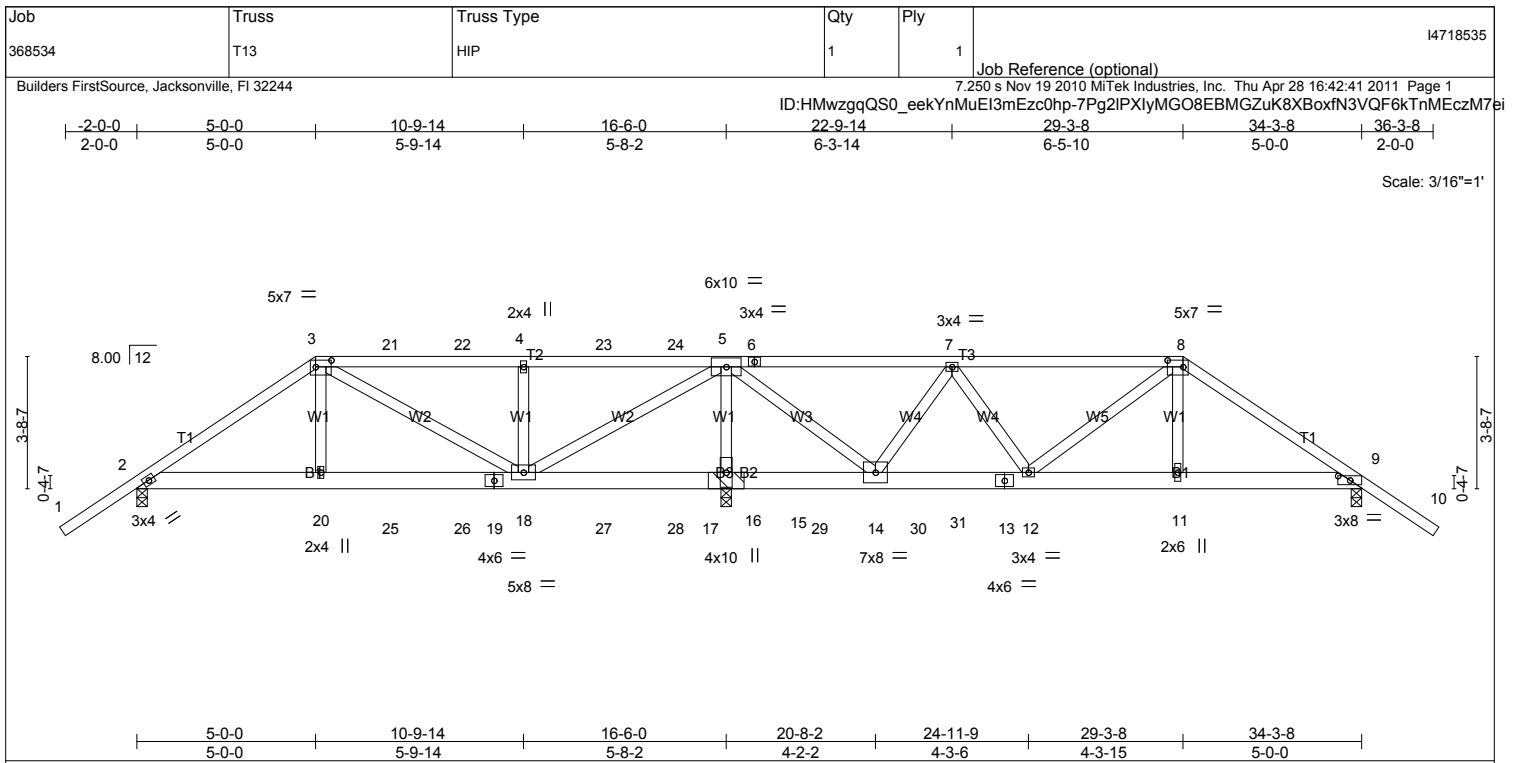
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=-266/162, 4-5=-266/162

- NOTES** (9-12)
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-05; 120mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - All bearings are assumed to be SYP No.2.
 - Refer to girder(s) for truss to truss connections.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5.
 - "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - For special connections with reactions or uplifts less than 300 lbs. Use typical toe-nail connection (refer to BFS detail package)
 - Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



April 28, 2011



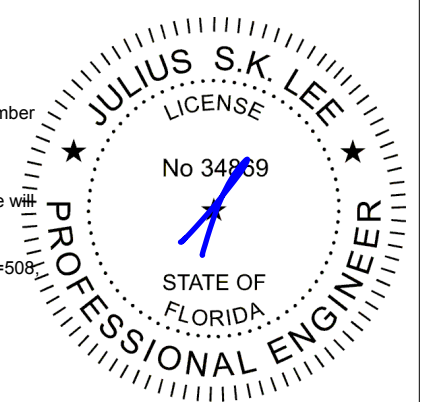
LOADING (psf)		SPACING		CSI		DEFL		PLATES		GRIP	
TCLL	20.0	Plates Increase	1.25	TC	0.43	Vert(LL)	0.10 12-14 >999 240	MT20		244/190	
TCDL	7.0	Lumber Increase	1.25	BC	0.55	Vert(TL)	-0.14 12-14 >999 180				
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.98	Horz(TL)	0.02 9 n/a n/a				
BCDL	5.0	Code FBC2007/TPI2002		(Matrix)							
								Weight: 206 lb		FT = 20%	

LUMBER		BRACING	
TOP CHORD	2 X 4 SYP No.2	TOP CHORD	Structural wood sheathing directly applied or 3-11-1 oc purlins.
BOT CHORD	2 X 6 SYP No.2	BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS	2 X 4 SYP No.3 *Except* W3: 2 X 4 SYP No.2		MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=613/0-3-8, 16=3830/0-4-8 (0-3-8 + bearing block), 9=1650/0-3-8
Max Horz 2=-109(LC 6)
Max Uplift 2=-508(LC 5), 16=-2700(LC 4), 9=-998(LC 6)
Max Grav 2=615(LC 9), 16=3830(LC 1), 9=1651(LC 10)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-674/603, 3-21=-93/319, 21-22=-92/319, 4-22=-92/318, 4-23=-92/319, 23-24=-92/319, 5-24=-92/319, 5-6=-1264/804, 6-7=-1264/804, 7-8=-1924/1218, 8-9=-2457/1549
BOT CHORD 2-20=-514/494, 20-25=-512/499, 25-26=-512/499, 19-26=-512/499, 18-19=-512/499, 18-27=-1315/966, 27-28=-1315/966, 17-28=-1315/966, 16-17=-1315/966, 15-16=-1315/966, 15-29=-1315/966, 14-29=-1315/966, 14-30=-986/1707, 30-31=-986/1707, 13-31=-986/1707, 12-13=-986/1707, 11-12=-1215/2013, 9-11=-1185/1966
WEBS 3-20=0/332, 3-18=-473/285, 4-18=-578/684, 5-18=-1351/1623, 5-16=-3277/2336, 5-14=-2089/3290, 7-14=-821/518, 7-12=-267/404, 8-11=-694/1066

NOTES (12-15)
1) 2 X 6 SYP No.2 bearing block 12" long at jt. 16 attached to front face with 3 rows of 10d (0.131"x3") nails spaced 3" o.c. 12 Total fasteners. Bearing is assumed to be SYP No.2.
2) Unbalanced roof live loads have been considered for this design.
3) Wind: ASCE 7-05; 120mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60
4) Provide adequate drainage to prevent water ponding.
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
7) All bearings are assumed to be SYP No.2.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=508, 16=2700, 9=998.
9) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.



Job	Truss	Truss Type	Qty	Ply	
368534	T13	HIP	1	1	14718535

Builders FirstSource, Jacksonville, FL 32244

Job Reference (optional)

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ID:HMwzgqQS0_eeKYNmuEI3mEzc0hp-7Pg2IPXlyMGO8EBMGZuK8XBoxfN3VQF6kTnMEczM7ei

NOTES (12-15)

- 10) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 167 lb down and 293 lb up at 5-0-1, 75 lb down and 160 lb up at 7-0-12, 75 lb down and 160 lb up at 9-0-12, 75 lb down and 160 lb up at 11-0-12, 75 lb down and 160 lb up at 13-0-12, and 75 lb down and 160 lb up at 15-0-12, and 98 lb down and 144 lb up at 29-3-7 on top chord, and 109 lb down at 5-0-0, 42 lb down at 7-0-12, 42 lb down at 9-0-12, 42 lb down at 11-0-12, 42 lb down at 13-0-12, 42 lb down at 15-0-12, 333 lb down and 270 lb up at 17-0-12, 333 lb down and 270 lb up at 19-0-12, 333 lb down and 270 lb up at 21-0-12, and 909 lb down and 520 lb up at 21-6-0, and 1105 lb down and 772 lb up at 29-2-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 11) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- 12) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
- 13) For special connections with reactions or uplifts less than 300 lbs. Use typical toe-nail connection (refer to BFS detail package)
- 14) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard

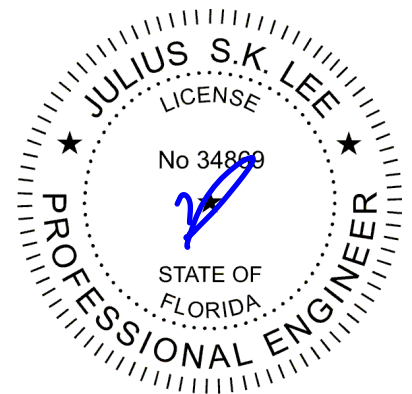
- 1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-3=-54, 3-8=-54, 8-10=-54, 2-9=-10

Concentrated Loads (lb)

Vert: 3=-127(F) 8=-58(F) 20=-33(F) 18=-14(F) 4=-75(F) 11=-1105(F) 15=-333(F) 21=-75(F) 22=-75(F) 23=-75(F) 24=-75(F) 25=-14(F) 26=-14(F) 27=-14(F) 28=-14(F) 29=-333(F) 30=-333(F) 31=-909(F)



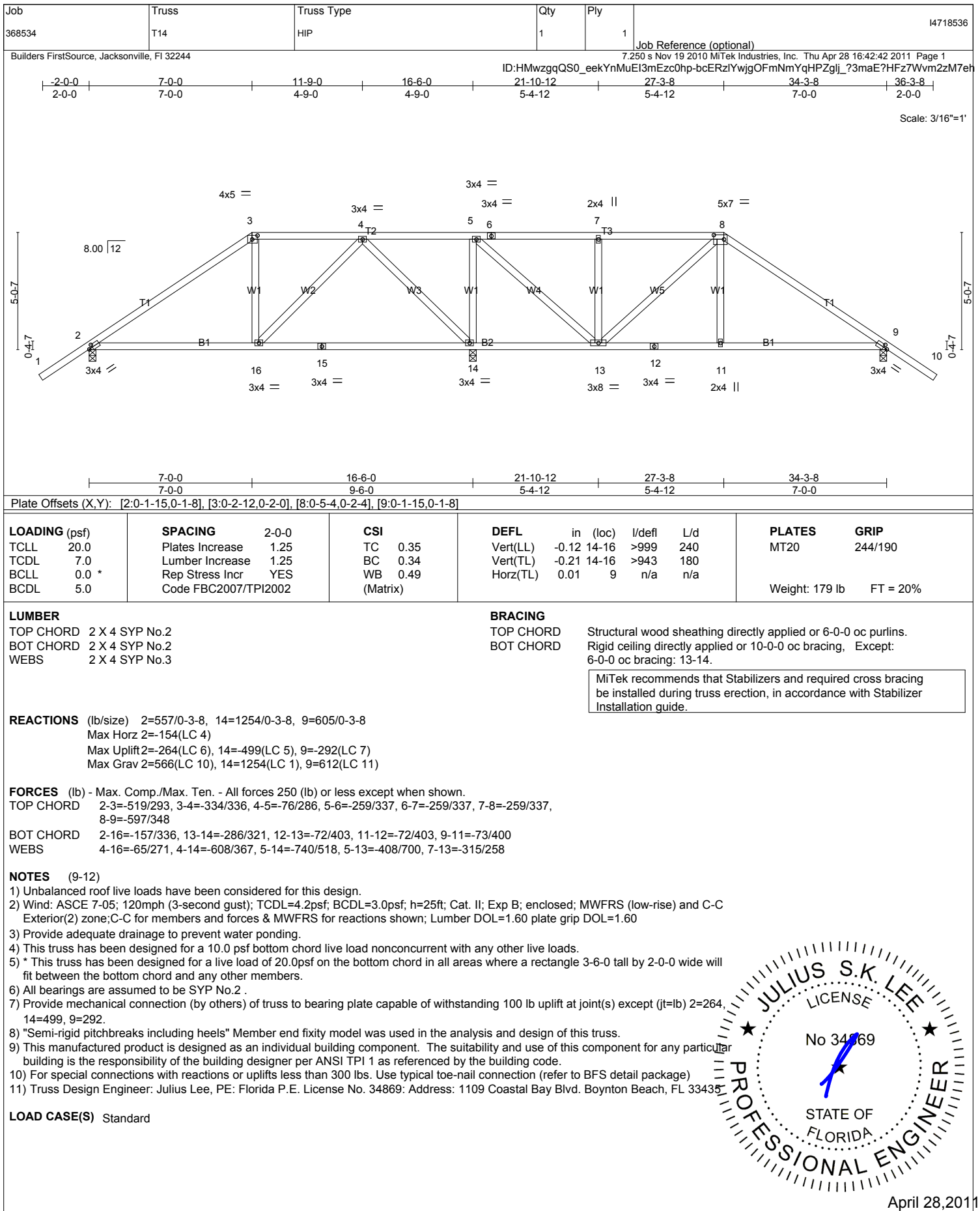
April 28, 2011



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435



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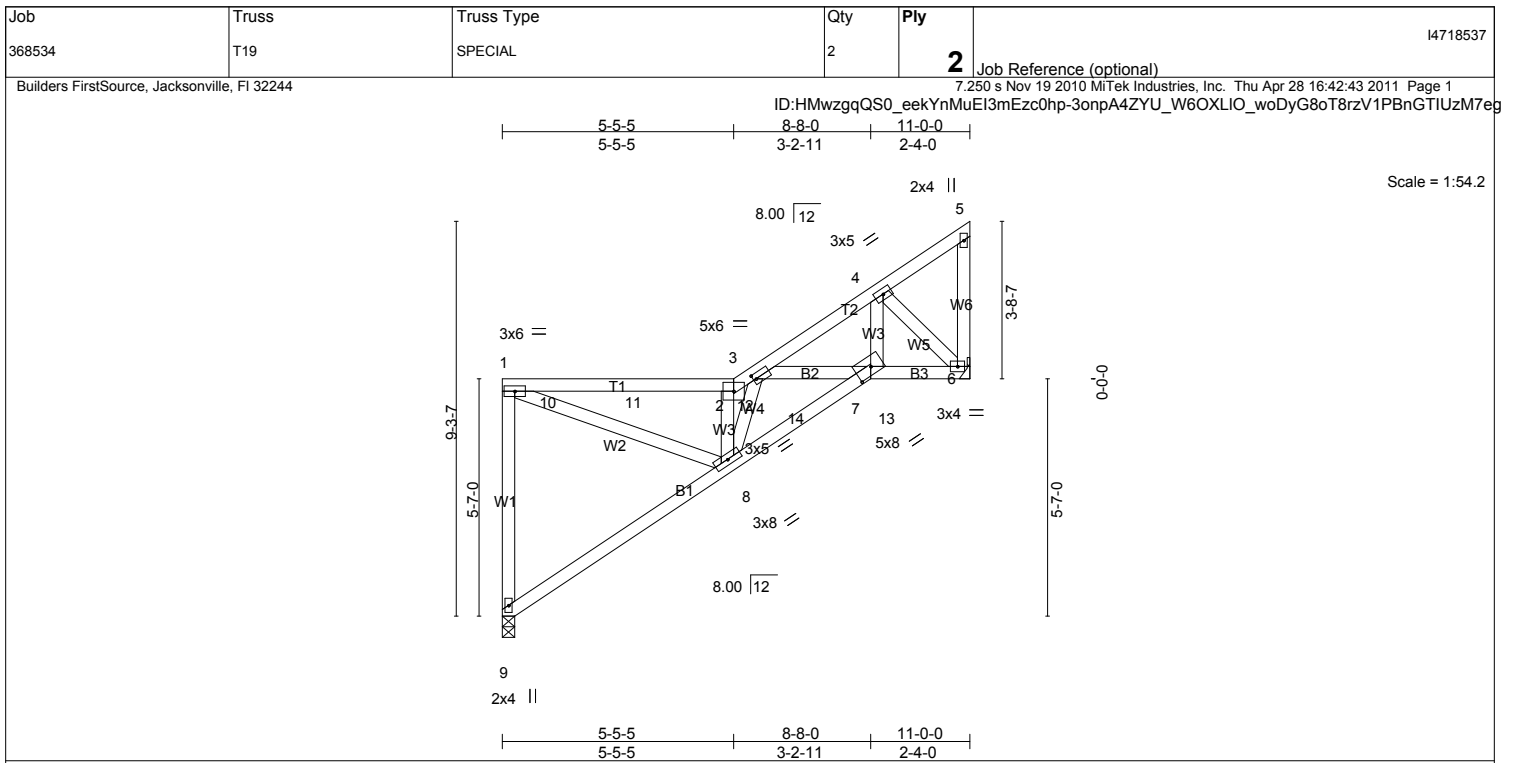


Plate Offsets (X,Y): [3:0-0-12,0-1-8], [7:0-4-8,0-2-4]											
LOADING (psf)		SPACING		CSI		DEFL		PLATES		GRIP	
TCLL	20.0	Plates Increase	1.25	TC	0.41	Vert(LL)	0.04 8 >999 240	MT20		244/190	
TCDL	7.0	Lumber Increase	1.25	BC	0.21	Vert(TL)	-0.05 8 >999 180				
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.26	Horz(TL)	0.03 6 n/a n/a				
BCDL	5.0	Code FBC2007/TPI2002		(Matrix)					Weight: 145 lb	FT = 20%	
LUMBER						BRACING					
TOP CHORD 2 X 4 SYP No.2						TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.					
BOT CHORD 2 X 4 SYP No.2						BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.					
WEBS 2 X 4 SYP No.3											
REACTIONS (lb/size) 9=1324/0-3-8, 6=919/Mechanical											
Max Horz 9=137(LC 5)											
Max Uplift 9=1086(LC 3), 6=502(LC 5)											
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.											
TOP CHORD 1-9=-1310/1142, 1-10=-1568/844, 10-11=-1569/845, 11-12=-1570/846, 2-12=-1571/846, 2-3=-2197/1285, 3-4=-1449/670											
BOT CHORD 7-8=-1203/2013, 7-13=-475/857, 6-13=-475/857, 3-14=-466/332, 7-14=-466/332											
WEBS 1-8=-871/1639, 2-8=-2027/1354, 4-7=-773/1479, 4-6=-1164/647, 3-8=-551/343											
NOTES (13-17)											
1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:											
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.											
Bottom chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.											
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.											
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.											
3) Wind: ASCE 7-05; 120mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60											
4) Provide adequate drainage to prevent water ponding.											
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.											
6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.											
7) All bearings are assumed to be SYP No.2 .											
8) Refer to girder(s) for truss to truss connections.											
9) Bearing at joint(s) 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.											
10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 9=1086, 6=502.											
11) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.											
12) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 299 lb down and 387 lb up at 0-1-12, 216 lb down and 365 lb up at 1-1-1, and 258 lb down and 240 lb up at 3-0-12, and 313 lb down and 195 lb up at 5-0-12 on top chord, and 223 lb down and 78 lb up at 9-0-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.											

JULIUS S.K. LEE

LICENSE

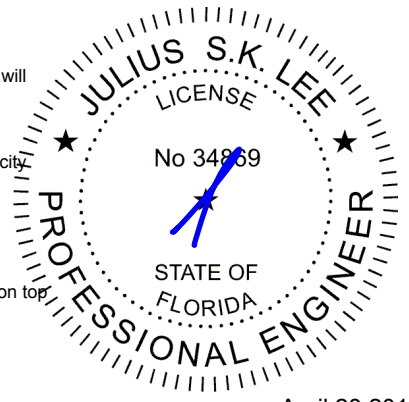
No 34869

STATE OF FLORIDA

PROFESSIONAL ENGINEER

Continued on page 2

April 28,2011



Job	Truss	Truss Type	Qty	Ply	
368534	T19	SPECIAL	2	2	

I4718537

Builders FirstSource, Jacksonville, FL 32244

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ID:HMwzqQS0_eeKynMuEI3mEzc0hp-3onpA4ZYU_W6OXLIO_woDyG8oT8rzV1PBnGTIUzM7eg

13) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.

14) For special connections with reactions or uplifts less than 300 lbs. Use typical toe-nail connection (refer to BFS detail package)

15) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

16) Use Simpson HHUS26-2 to attach Truss to Carrying member

LOAD CASE(S) Standard

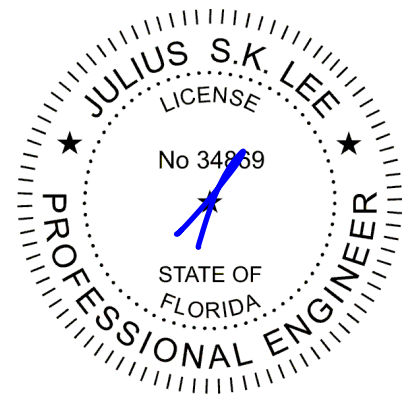
1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-2=-54, 2-5=-54, 7-9=-10, 6-7=-10, 3-7=-10

Concentrated Loads (lb)

Vert: 1=-299 10=-216 11=-258 12=-313 13=-223(F) 14=-223(F)



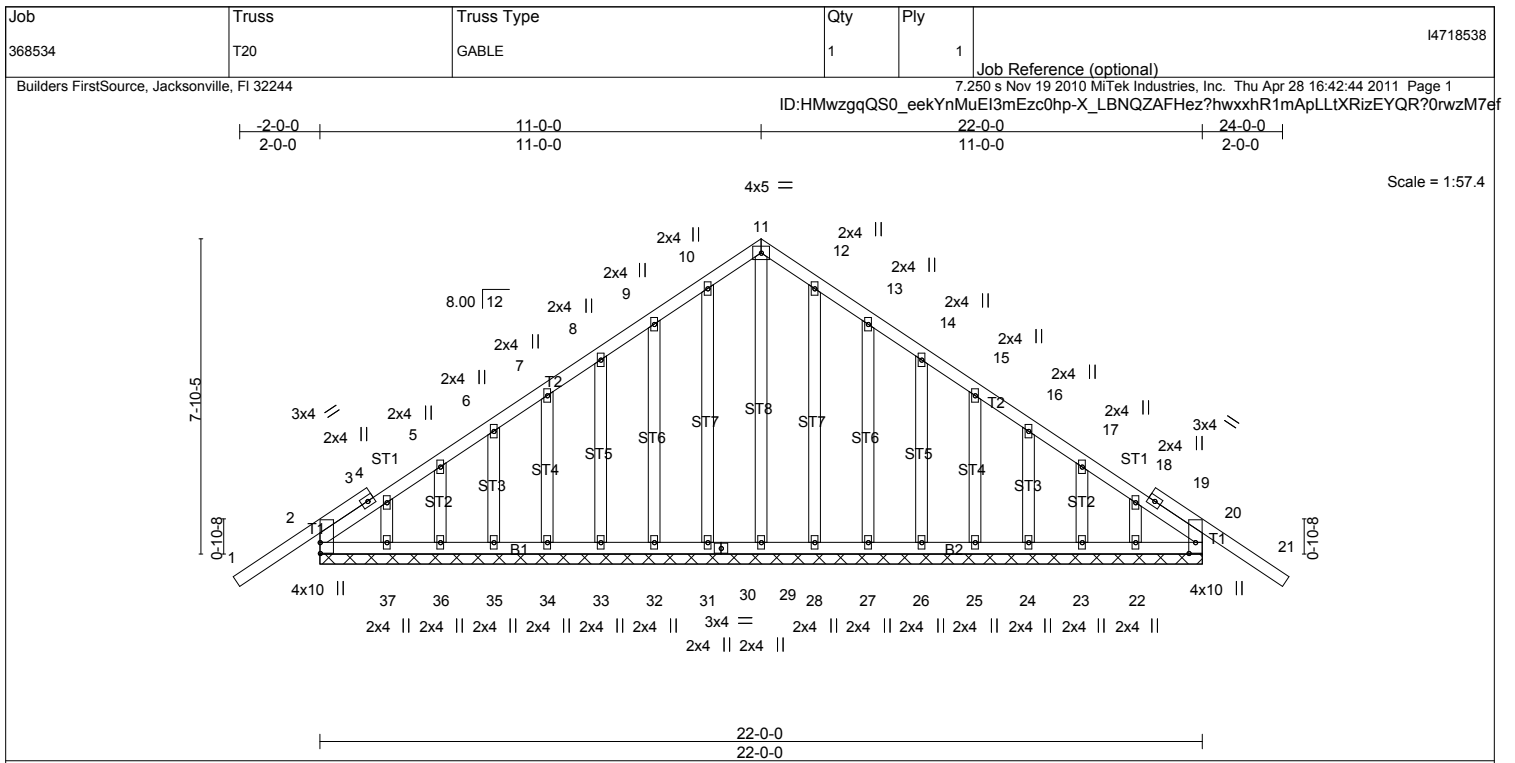
April 28, 2011



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Boynton, FL 33435



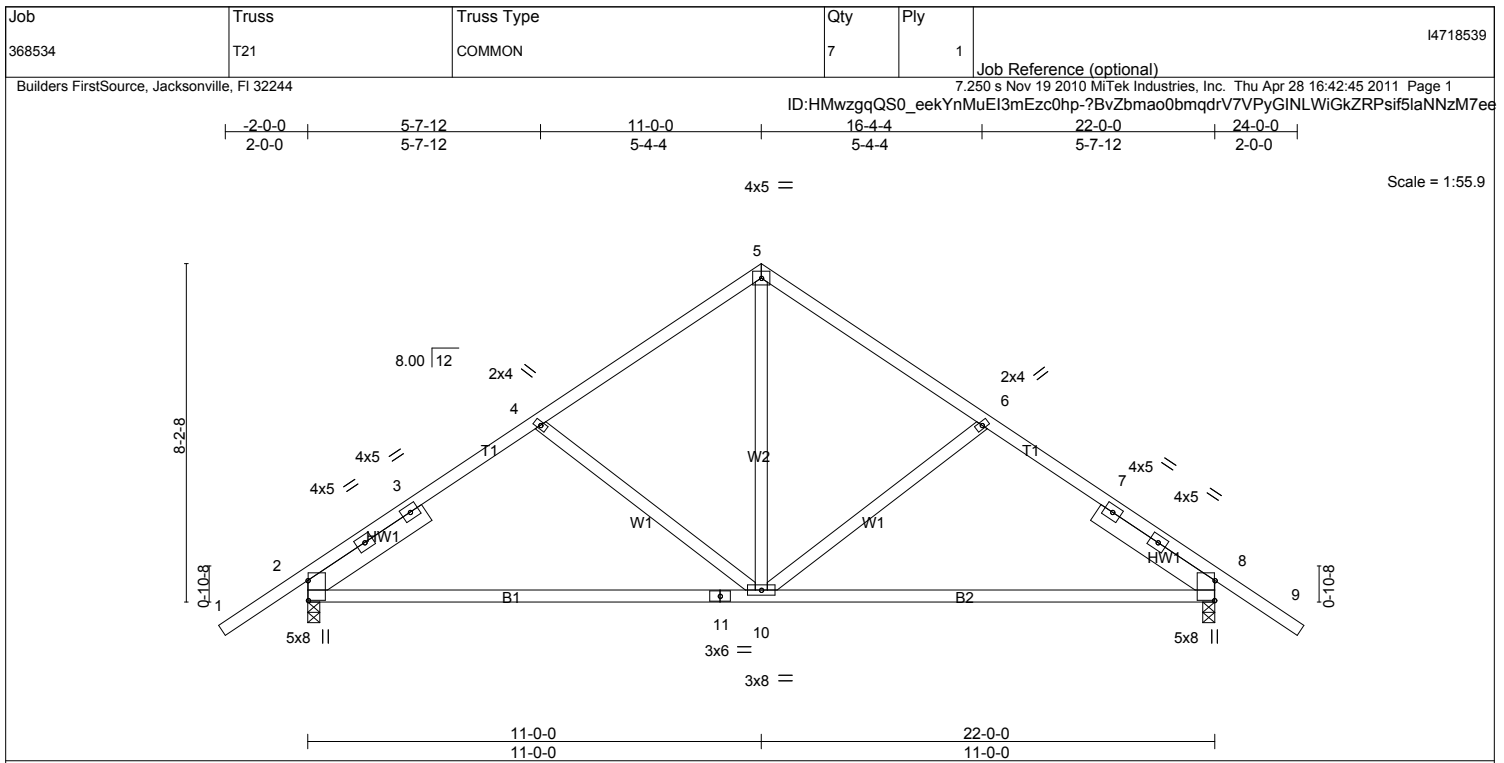


Plate Offsets (X,Y): [2:0-5-13,0-0-2], [8:0-5-13,0-0-2]					
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	PLATES
TCLL	Plates Increase	1.25	TC	0.26	GRIP
TCDL	Lumber Increase	1.25	BC	0.58	
BCCL	Rep Stress Incr	YES	WB	0.30	
BCDL	Code FBC2007/TPI2002		(Matrix)		
				Weight: 127 lb	FT = 20%

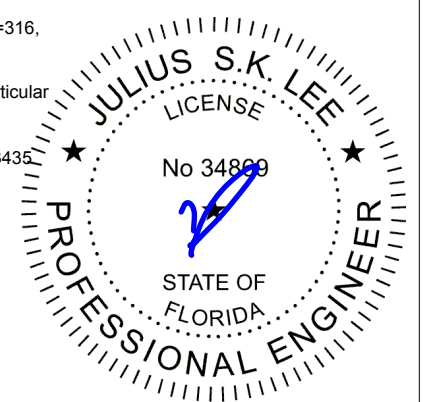
LUMBER		BRACING	
TOP CHORD	2 X 4 SYP No.2	TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD	2 X 4 SYP No.2	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS	2 X 4 SYP No.3		
SLIDER	Left 2 X 6 SYP No.2 3-5-5, Right 2 X 6 SYP No.2 3-5-5		

REACTIONS (lb/size) 2=816/0-3-8, 8=816/0-3-8
Max Horz 2=259(LC 5)
Max Uplift 2=316(LC 6), 8=316(LC 7)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-933/512, 3-4=-838/536, 4-5=-686/469, 5-6=-686/469, 6-7=-838/536, 7-8=-933/512
BOT CHORD 2-11=-226/667, 10-11=-226/667, 8-10=-226/667
WEBS 5-10=-279/435, 6-10=-231/290, 4-10=-231/290

- NOTES** (8-11)
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-05; 120mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - All bearings are assumed to be SYP No.2 .
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=316, 8=316.
 - "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - For special connections with reactions or uplifts less than 300 lbs. Use typical toe-nail connection (refer to BFS detail package)
 - Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



April 28, 2011

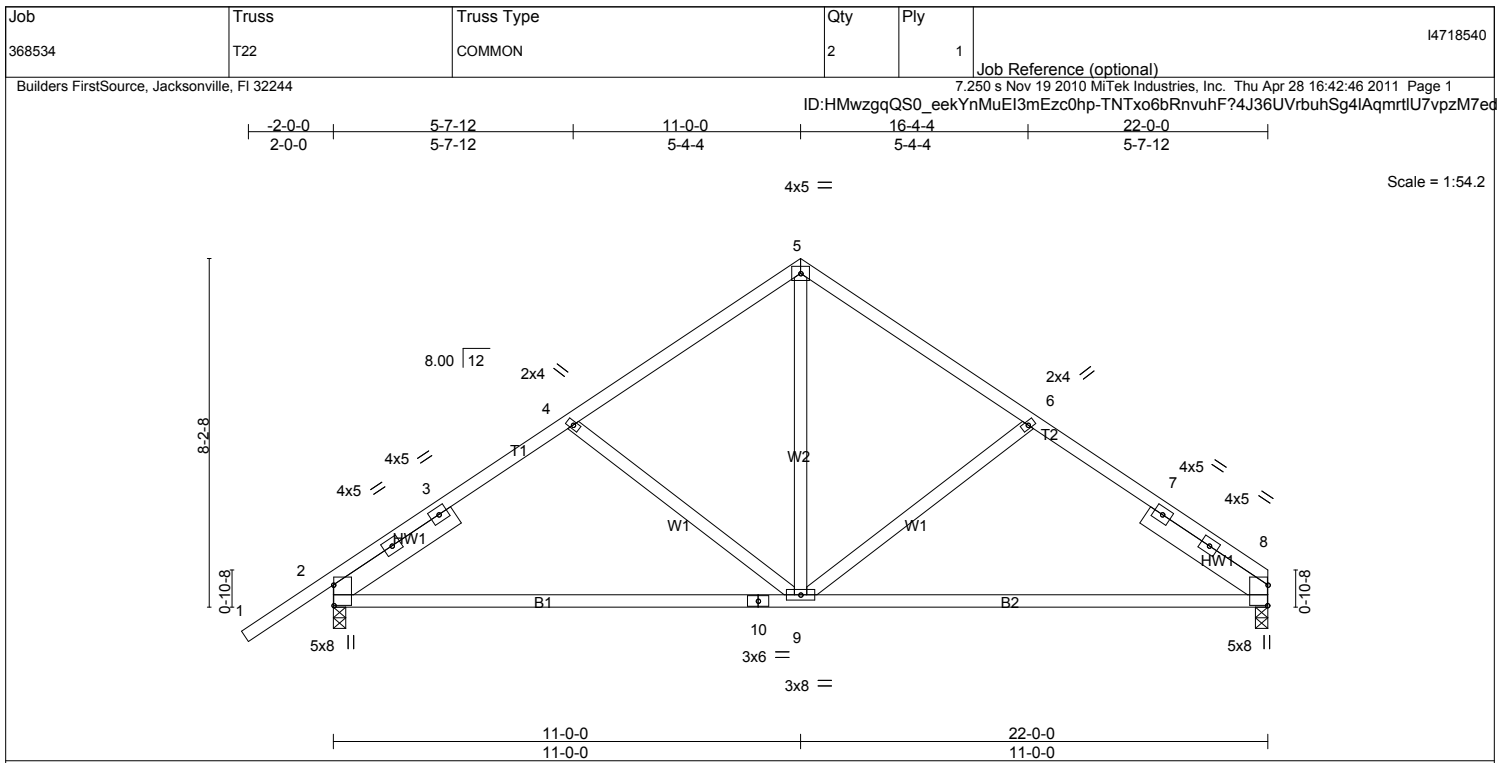


Plate Offsets (X,Y): [2:0-5-13,0-0-2], [8:0-5-13,0-0-2]					
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	PLATES
TCLL	Plates Increase	1.25	TC	0.26	MT20
TCDL	Lumber Increase	1.25	BC	0.58	244/190
BCLL	Rep Stress Incr	YES	WB	0.33	
BCDL	Code FBC2007/TPI2002		(Matrix)		
			Weight: 123 lb		FT = 20%

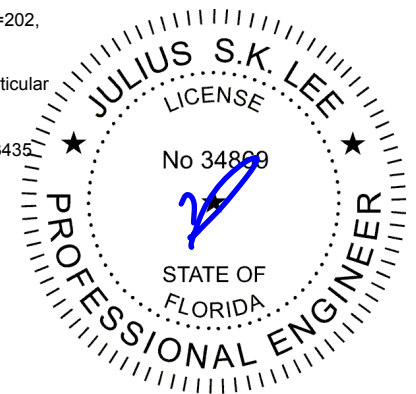
LUMBER		BRACING	
TOP CHORD	2 X 4 SYP No.2	TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD	2 X 4 SYP No.2	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS	2 X 4 SYP No.3		
SLIDER	Left 2 X 6 SYP No.2 3-5-5, Right 2 X 6 SYP No.2 3-5-5		

REACTIONS (lb/size) 8=699/0-3-8, 2=822/0-3-8
Max Horz 2=279(LC 5)
Max Uplift 8=202(LC 7), 2=-317(LC 6)

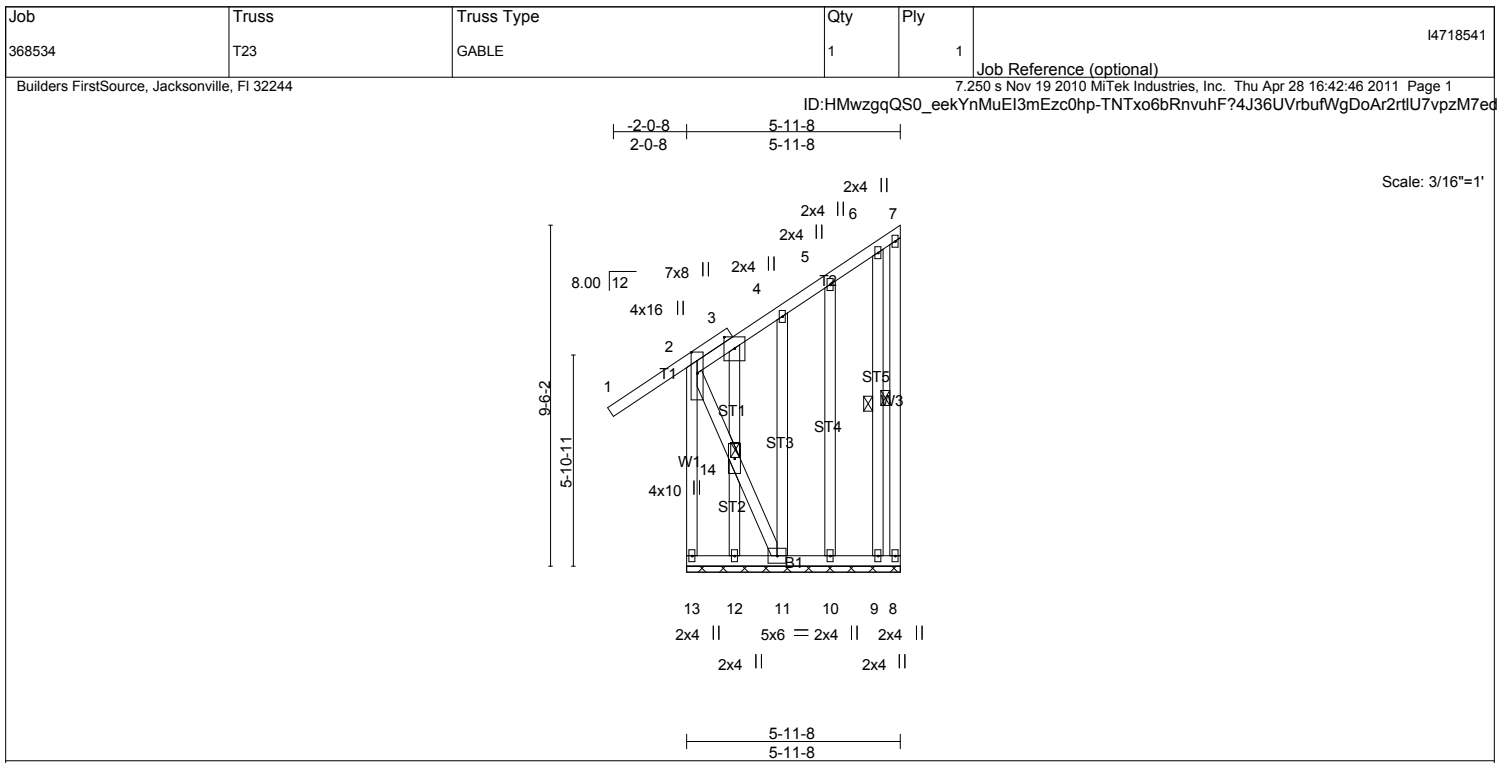
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-942/526, 3-4=-847/550, 4-5=-695/483, 5-6=-697/486, 6-7=-787/563, 7-8=-929/542
BOT CHORD 2-10=-303/674, 9-10=-303/674, 8-9=-329/690
WEBS 5-9=-300/438, 6-9=-252/322, 4-9=-231/290

- NOTES** (8-11)
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-05; 120mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - All bearings are assumed to be SYP No.2.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 8=202, 2=317.
 - "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - For special connections with reactions or uplifts less than 300 lbs. Use typical toe-nail connection (refer to BFS detail package)
 - Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

LOAD CASE(S) Standard



April 28, 2011



LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.44	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Plates Increase 1.25	BC 0.01	Vert(LL) 0.00 1 n/r 120		
BCLL 0.0 *	Lumber Increase 1.25	WB 0.31	Vert(TL) -0.02 1 n/r 90		
BCDL 5.0	Rep Stress Incr NO	(Matrix)	Horz(TL) -0.00 8 n/a n/a		
	Code FBC2007/TPI2002			Weight: 98 lb	FT = 20%

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 5-11-8 oc purlins, except end verticals.
BOT CHORD 2 X 4 SYP M 31	BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 2 X 4 SYP M 31 *Except*	WEBS 1 Row at midpt 7-8, 6-9
W3: 2 X 4 SYP No.2, W2: 2 X 4 SYP No.3	JOINTS 1 Brace at Jt(s): 14
OTHERS 2 X 4 SYP No.3	

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS All bearings 5-11-8.

(lb) - Max Horz 13=451(LC 6)

Max Uplift All uplift 100 lb or less at joint(s) 8, 10, 9 except 13=194(LC 4), 11=895(LC 6)

Max Grav All reactions 250 lb or less at joint(s) 8, 12, 11, 10, 9 except 13=542(LC 6)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-13=-723/197

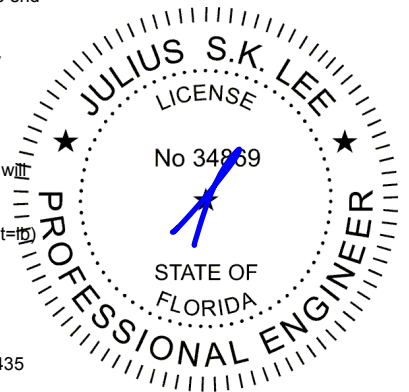
BOT CHORD 12-13=-488/62, 11-12=-488/62

WEBS 2-14=-144/1147, 11-14=-149/1159

- NOTES** (10-13)
- Wind: ASCE 7-05; 120mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; end vertical left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002.
 - Gable requires continuous bottom chord bearing.
 - Gable studs spaced at 1-4-0 oc.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - All bearings are assumed to be SYP No.2 .
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 8, 10, 9 except (jt=13)=194, 11=895.
 - "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - For special connections with reactions or uplifts less than 300 lbs. Use typical toe-nail connection (refer to BFS detail package)
 - Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

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April 28,2011



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Julius Lee
1109 Coastal Bay Blvd.
Boynton, FL 33435

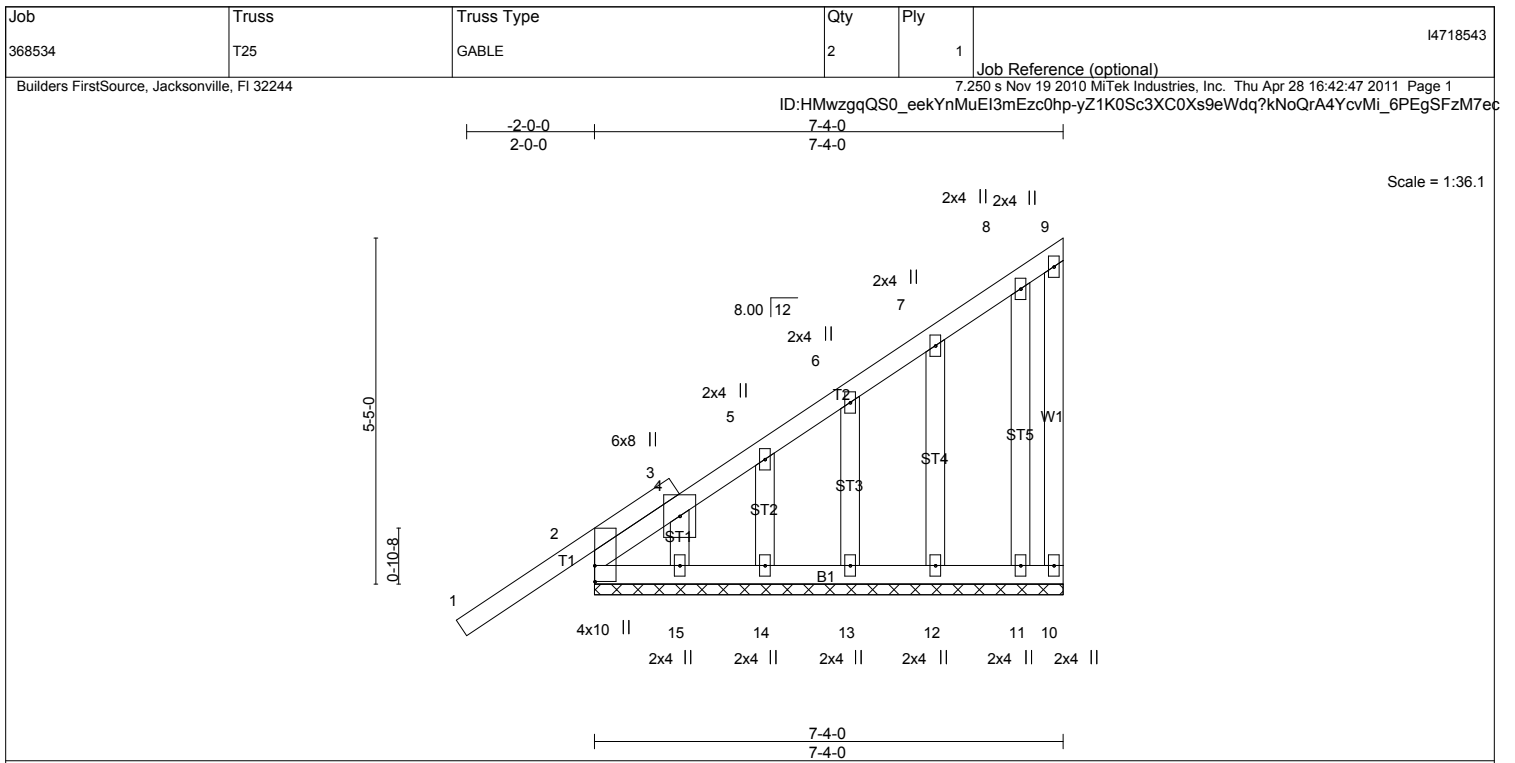


Plate Offsets (X,Y): [2:0-3-0,0-0-1]							
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	L/defl	L/d
TCLL 20.0	Plates Increase	1.25	TC 0.32	Vert(LL)	0.00	1	n/r
TCDL 7.0	Lumber Increase	1.25	BC 0.03	Vert(TL)	-0.01	1	n/r
BCLL 0.0 *	Rep Stress Incr	NO	WB 0.03	Horz(TL)	-0.00	10	n/a
BCDL 5.0	Code FBC2007/TPI2002		(Matrix)				
				PLATES	GRIP		
				MT20	244/190		
				Weight: 57 lb		FT = 20%	

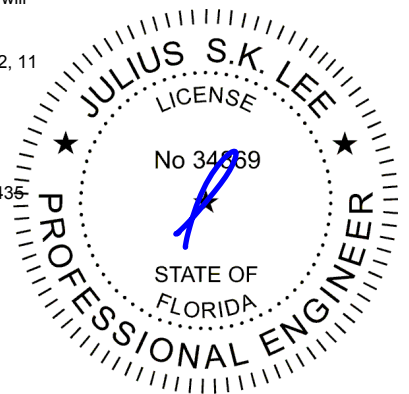
LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD 2 X 4 SYP No.2	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 2 X 4 SYP No.3	
OTHERS 2 X 4 SYP No.3	
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.	

REACTIONS All bearings 7-4-0.
(lb) - Max Horz 2=337(LC 6)
Max Uplift All uplift 100 lb or less at joint(s) 10, 15, 14, 13, 12, 11 except 2=103(LC 6)
Max Grav All reactions 250 lb or less at joint(s) 2, 10, 15, 14, 13, 12, 11

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- NOTES** (10-13)
- Wind: ASCE 7-05; 120mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002.
 - Gable requires continuous bottom chord bearing.
 - Gable studs spaced at 1-4-0 oc.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - All bearings are assumed to be SYP No.2.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 10, 15, 14, 13, 12, 11 except (jt=lb) 2=103.
 - "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - For special connections with reactions or uplifts less than 300 lbs. Use typical toe-nail connection (refer to BFS detail package)
 - Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

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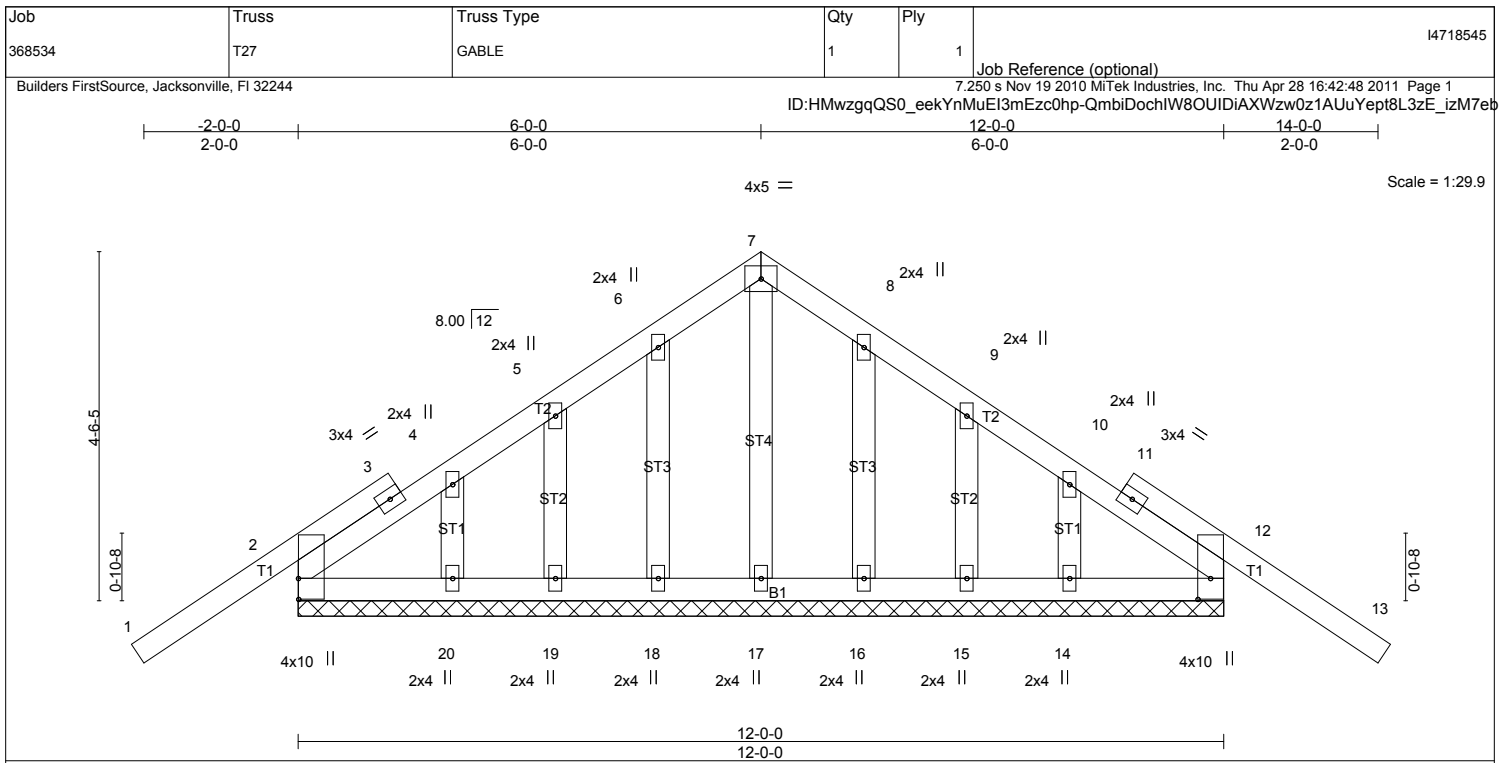


Plate Offsets (X,Y): [2:0-3-4,0-0-1], [12:0-3-4,0-1-15]					
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc) l/defl L/d
TCLL	20.0	Plates Increase	1.25	TC	0.31
TCDL	7.0	Lumber Increase	1.25	BC	0.05
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.03
BCDL	5.0	Code FBC2007/TPI2002	(Matrix)	Horz(TL)	0.00 12 n/a n/a
			PLATES		GRIP
			MT20		244/190
			Weight: 77 lb		FT = 20%

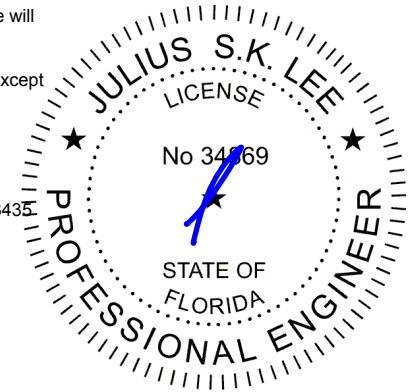
LUMBER		BRACING	
TOP CHORD 2 X 4 SYP No.2		TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.	
BOT CHORD 2 X 4 SYP No.2		BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.	
OTHERS 2 X 4 SYP No.3		MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.	

REACTIONS All bearings 12-0-0.
(lb) - Max Horz 2=179(LC 5)
Max Uplift All uplift 100 lb or less at joint(s) 18, 20, 16, 14 except 2=-213(LC 6), 12=-239(LC 7), 19=-111(LC 6), 15=-113(LC 7)
Max Grav All reactions 250 lb or less at joint(s) 2, 12, 17, 18, 19, 20, 16, 15, 14

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- NOTES** (11-14)
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-05; 120mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002.
 - Gable requires continuous bottom chord bearing.
 - Gable studs spaced at 1-4-0 oc.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - All bearings are assumed to be SYP No.2.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 18, 20, 16, 14 except (jt=lb) 2=213, 12=239, 19=111, 15=113.
 - "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - For special connections with reactions or uplifts less than 300 lbs. Use typical toe-nail connection (refer to BFS detail package)
 - Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

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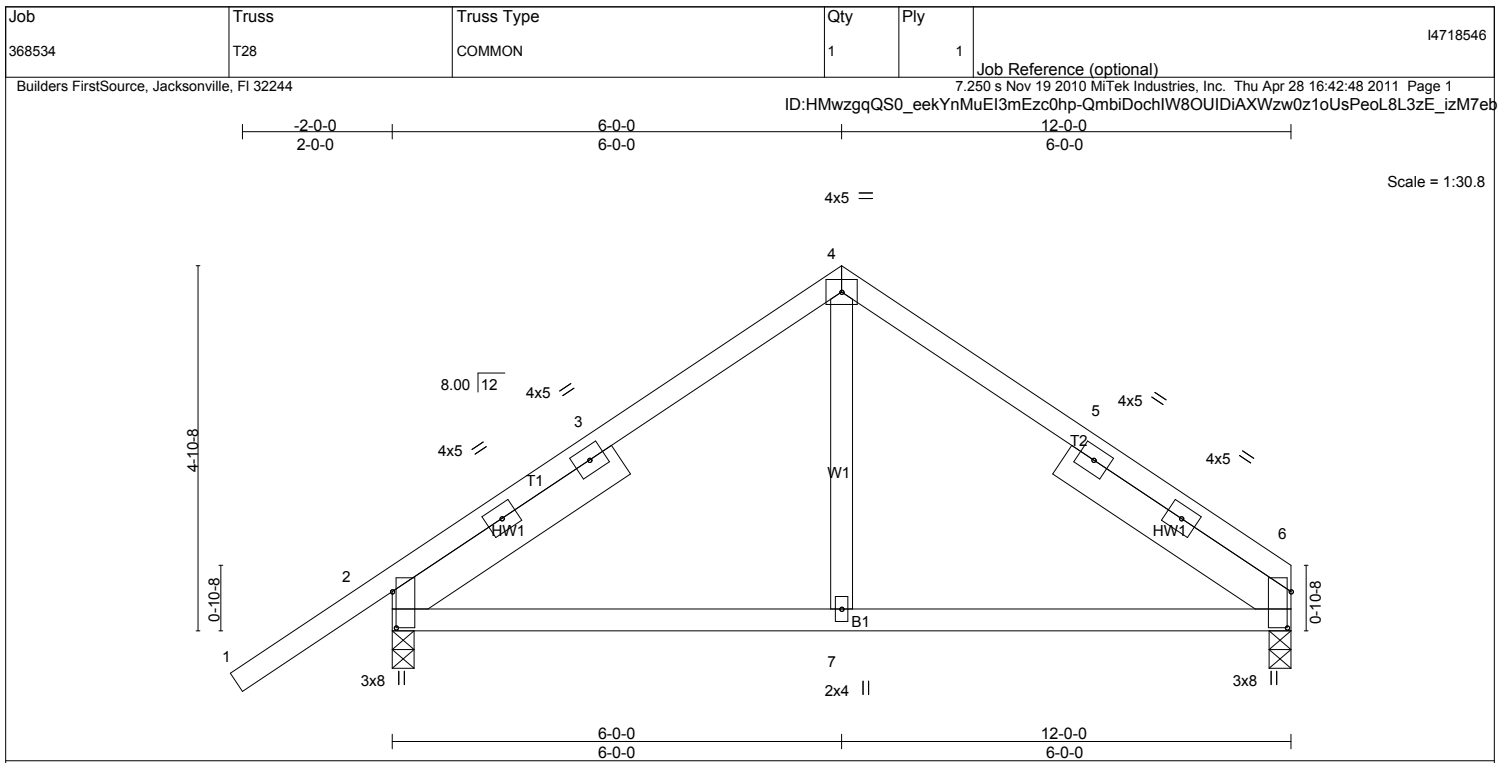


Plate Offsets (X,Y): [2:0-5-13,0-0-10], [6:0-5-13,0-0-10]									
LOADING (psf)		SPACING 2-0-0		CSI		DEFL		PLATES	
TCLL	20.0	Plates Increase	1.25	TC	0.27	in (loc)	I/defl	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.19	Vert(LL)	-0.02 6-7 >999	Weight: 67 lb FT = 20%	
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.07	Vert(TL)	-0.04 6-7 >999		
BCDL	5.0	Code FBC2007/TPI2002		(Matrix)		Horz(TL)	0.01 6 n/a n/a		

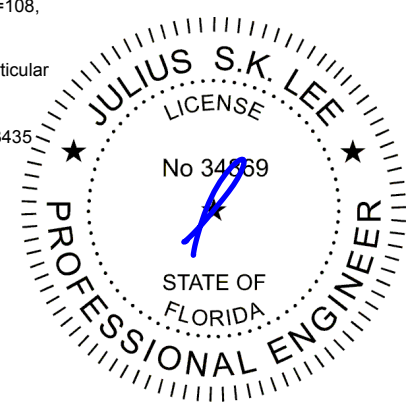
LUMBER		BRACING	
TOP CHORD	2 X 4 SYP No.2	TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD	2 X 4 SYP No.2	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS	2 X 4 SYP No.3	MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.	
SLIDER	Left 2 X 6 SYP No.2 3-7-12, Right 2 X 6 SYP No.2 3-7-12		

REACTIONS (lb/size) 6=374/0-3-8, 2=506/0-3-8
Max Horz 2=169(LC 5)
Max Uplift 6=108(LC 7), 2=-228(LC 6)

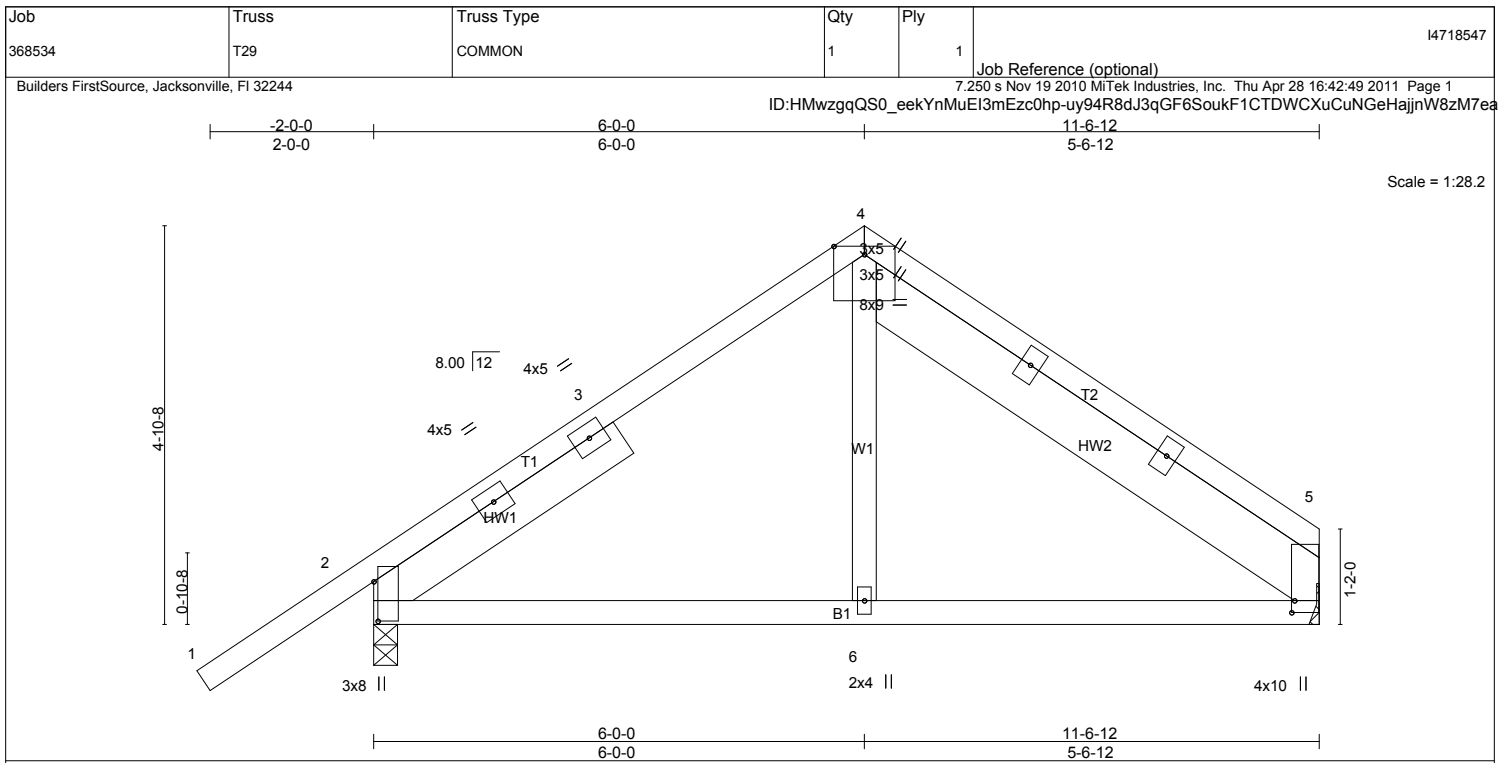
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-447/232, 3-4=-322/245, 4-5=-322/230, 5-6=-416/213
BOT CHORD 2-7=-41/268, 6-7=-41/268

- NOTES** (8-11)
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-05; 120mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - All bearings are assumed to be SYP No.2 .
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 6=108, 2=228.
 - "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - For special connections with reactions or uplifts less than 300 lbs. Use typical toe-nail connection (refer to BFS detail package)
 - Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

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LOADING (psf)		SPACING		CSI		DEFL		PLATES		GRIP	
TCLL	20.0	Plates Increase	1.25	TC	0.27	Vert(LL)	-0.02	MT20		244/190	
TCDL	7.0	Lumber Increase	1.25	BC	0.17	Vert(TL)	-0.04				
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.06	Horz(TL)	0.01				
BCDL	5.0	Code FBC2007/TPI2002		(Matrix)							
								Weight: 78 lb		FT = 20%	

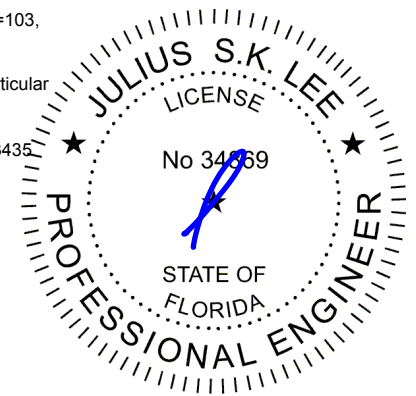
LUMBER		BRACING	
TOP CHORD	2 X 4 SYP No.2	TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD	2 X 4 SYP No.2	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS	2 X 4 SYP No.3		
SLIDER	Left 2 X 6 SYP No.2 3-7-12, Right 2 X 8 SYP No.1D 6-9-10		

REACTIONS (lb/size) 5=360/Mechanical, 2=492/0-3-8
Max Horz 2=155(LC 5)
Max Uplift 5=103(LC 7), 2=-228(LC 6)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-435/216, 3-4=-302/238, 4-5=-398/208
BOT CHORD 2-6=-50/267, 5-6=-51/266

- NOTES** (9-13)
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-05; 120mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - All bearings are assumed to be SYP No.2 .
 - Refer to girder(s) for truss to truss connections.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 5=103, 2=228.
 - "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - For special connections with reactions or uplifts less than 300 lbs. Use typical toe-nail connection (refer to BFS detail package)
 - Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435
 - Use Simpson HTU26 to attach Truss to Carrying member

LOAD CASE(S) Standard



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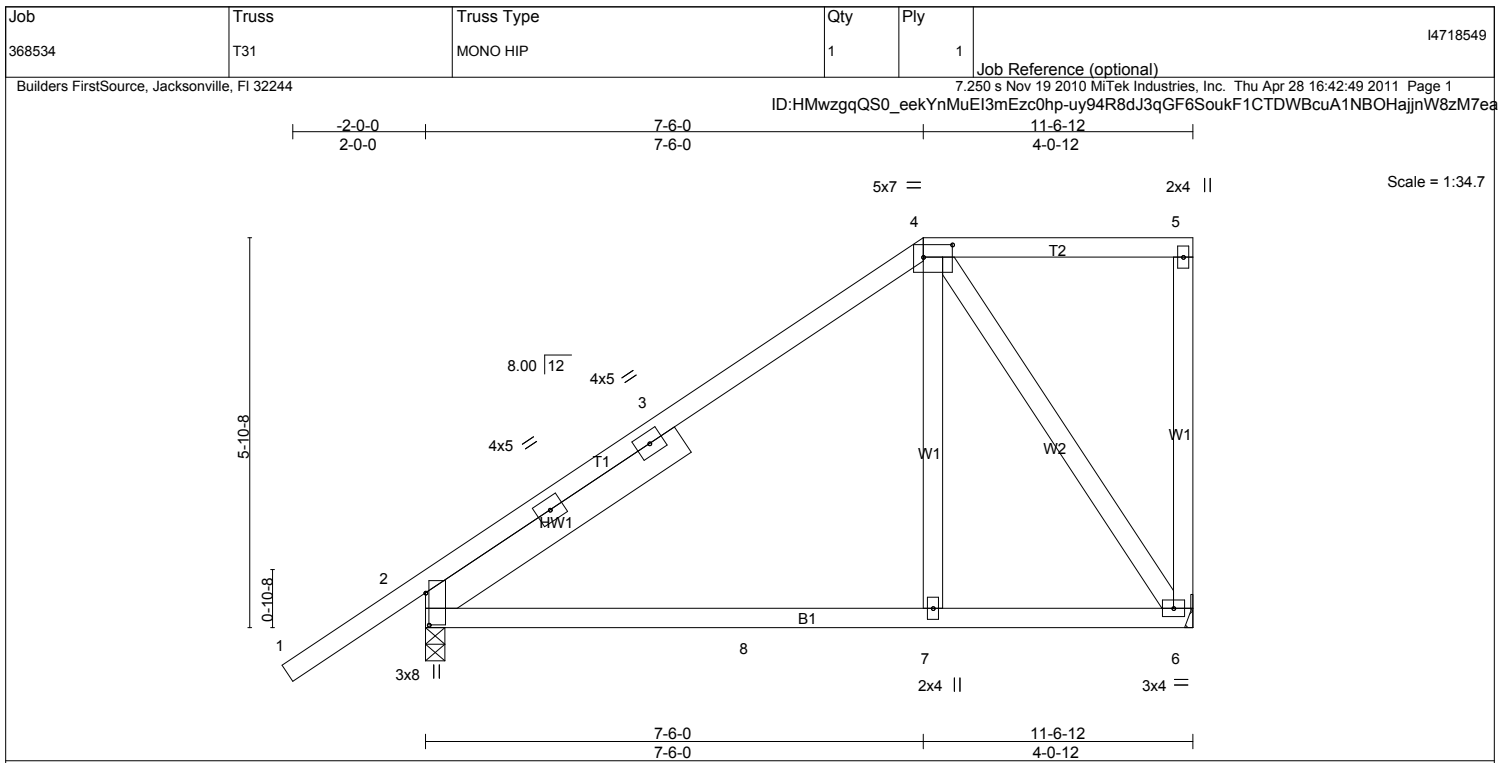


Plate Offsets (X,Y): [2:0-5-13,0-0-10], [4:0-5-4,0-2-4]									
LOADING (psf)	SPACING		CSI	DEFL	in	(loc)	I/defl	L/d	PLATES GRIP
TCLL 20.0	Plates Increase 1.25		TC 0.33	Vert(LL) -0.07	2-7	>999	240		MT20 244/190
TCDL 7.0	Lumber Increase 1.25		BC 0.29	Vert(TL) -0.14	2-7	>983	180		
BCLL 0.0 *	Rep Stress Incr YES		WB 0.33	Horz(TL) 0.00	6	n/a	n/a		
BCDL 5.0	Code FBC2007/TPI2002		(Matrix)						Weight: 76 lb FT = 20%

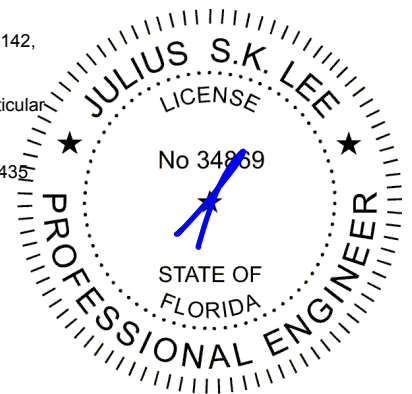
LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD 2 X 4 SYP No.2	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 2 X 4 SYP No.3	
SLIDER Left 2 X 6 SYP No.2 4-7-10	
	MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 6=415/Mechanical, 2=537/0-3-8
Max Horz 2=263(LC 6)
Max Uplift 6=142(LC 5), 2=194(LC 6)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-430/64, 3-4=-291/81
WEBS 4-6=-427/320

- NOTES** (9-13)
- Wind: ASCE 7-05; 120mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 5.0psf.
 - All bearings are assumed to be SYP No.2.
 - Refer to girder(s) for truss to truss connections.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 6=142, 2=194.
 - "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - For special connections with reactions or uplifts less than 300 lbs. Use typical toe-nail connection (refer to BFS detail package)
 - Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435
 - Use Simpson HTU26 to attach Truss to Carrying member

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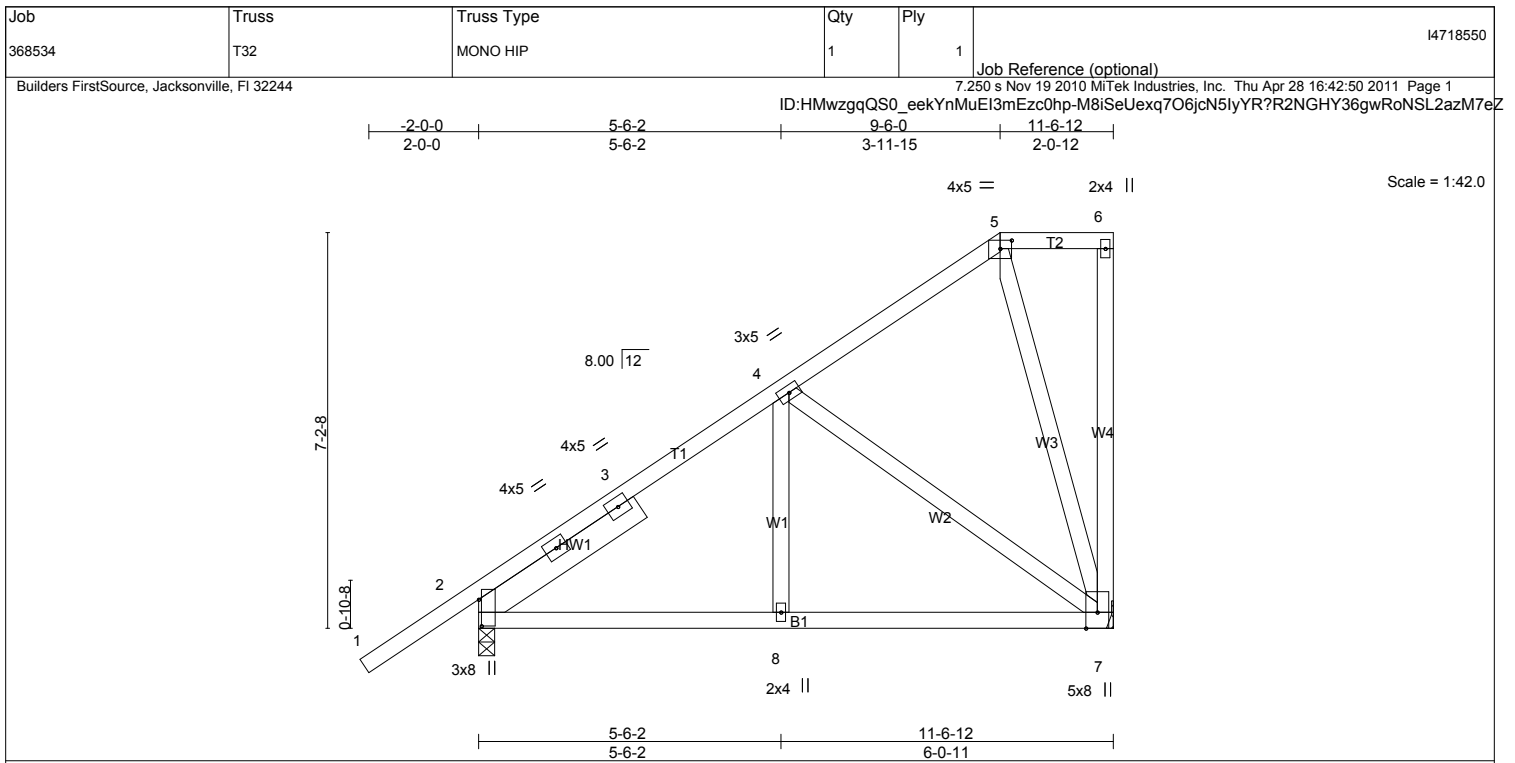


Plate Offsets (X,Y): [2:0-5-13,0-0-10], [5:0-2-8,0-1-13]					
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc) l/defl L/d
TCLL	20.0	Plates Increase	1.25	TC	0.27
TCDL	7.0	Lumber Increase	1.25	BC	0.18
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.25
BCDL	5.0	Code FBC2007/TPI2002	(Matrix)	Horz(TL)	-0.01 7 n/a n/a
			PLATES		GRIP
			MT20		244/190
			Weight: 85 lb		FT = 20%

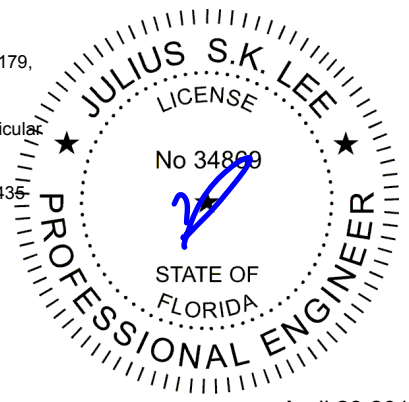
LUMBER		BRACING	
TOP CHORD	2 X 4 SYP No.2	TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD	2 X 4 SYP No.2	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS	2 X 4 SYP No.3	MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.	
SLIDER	Left 2 X 6 SYP No.2 3-6-3		

REACTIONS (lb/size) 7=355/Mechanical, 2=488/0-3-8
Max Horz 2=314(LC 6)
Max Uplift 7=179(LC 6), 2=173(LC 6)

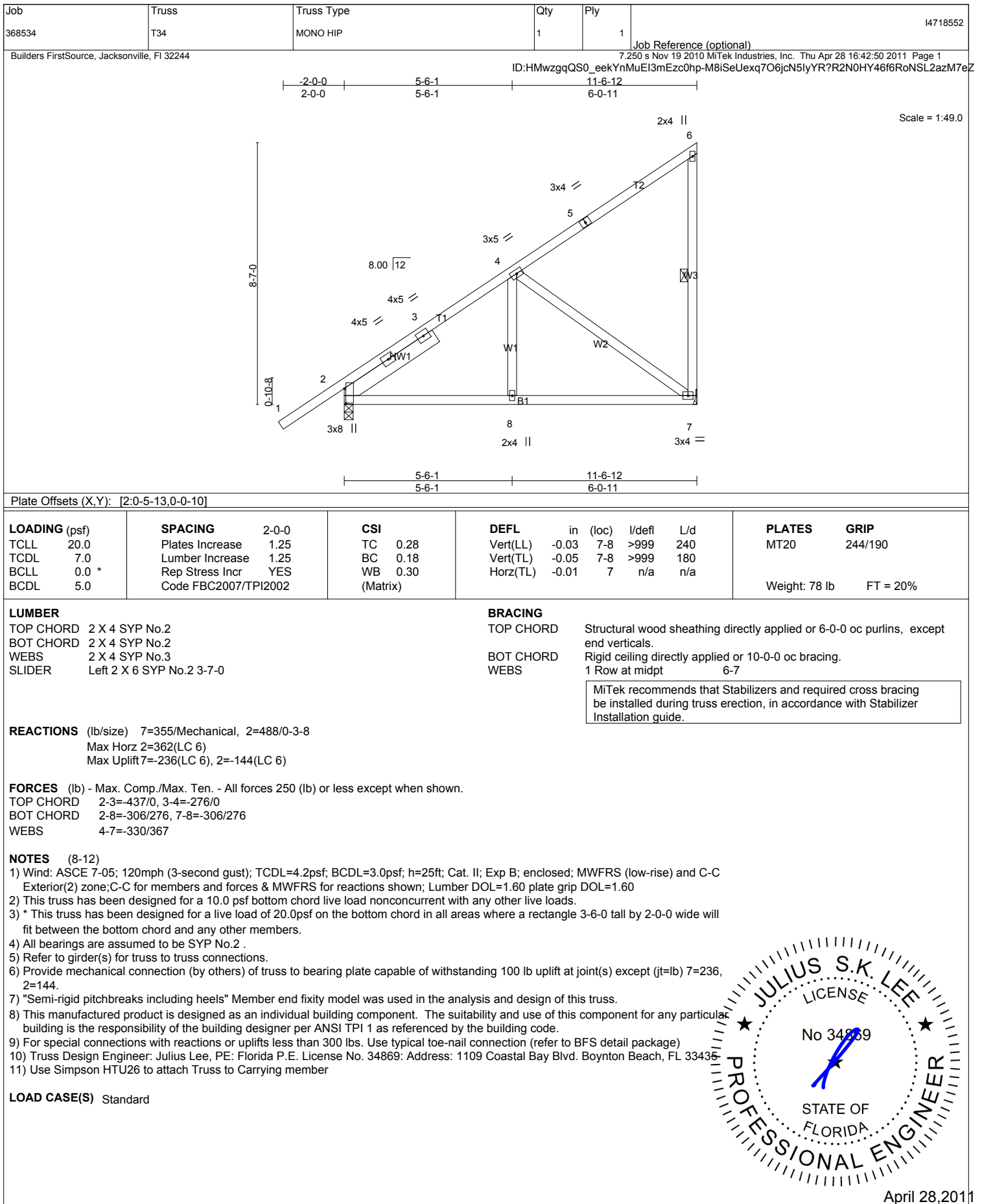
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-429/39, 3-4=-314/52
BOT CHORD 2-8=-280/261, 7-8=-280/261
WEBS 4-7=-274/291

- NOTES** (9-13)
- 1) Wind: ASCE 7-05; 120mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 2) Provide adequate drainage to prevent water ponding.
 - 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - 5) All bearings are assumed to be SYP No.2 .
 - 6) Refer to girder(s) for truss to truss connections.
 - 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 7=179, 2=173.
 - 8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - 10) For special connections with reactions or uplifts less than 300 lbs. Use typical toe-nail connection (refer to BFS detail package)
 - 11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435
 - 12) Use Simpson HTU26 to attach Truss to Carrying member

LOAD CASE(S) Standard



April 28, 2011



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.

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Boynton, FL 33435

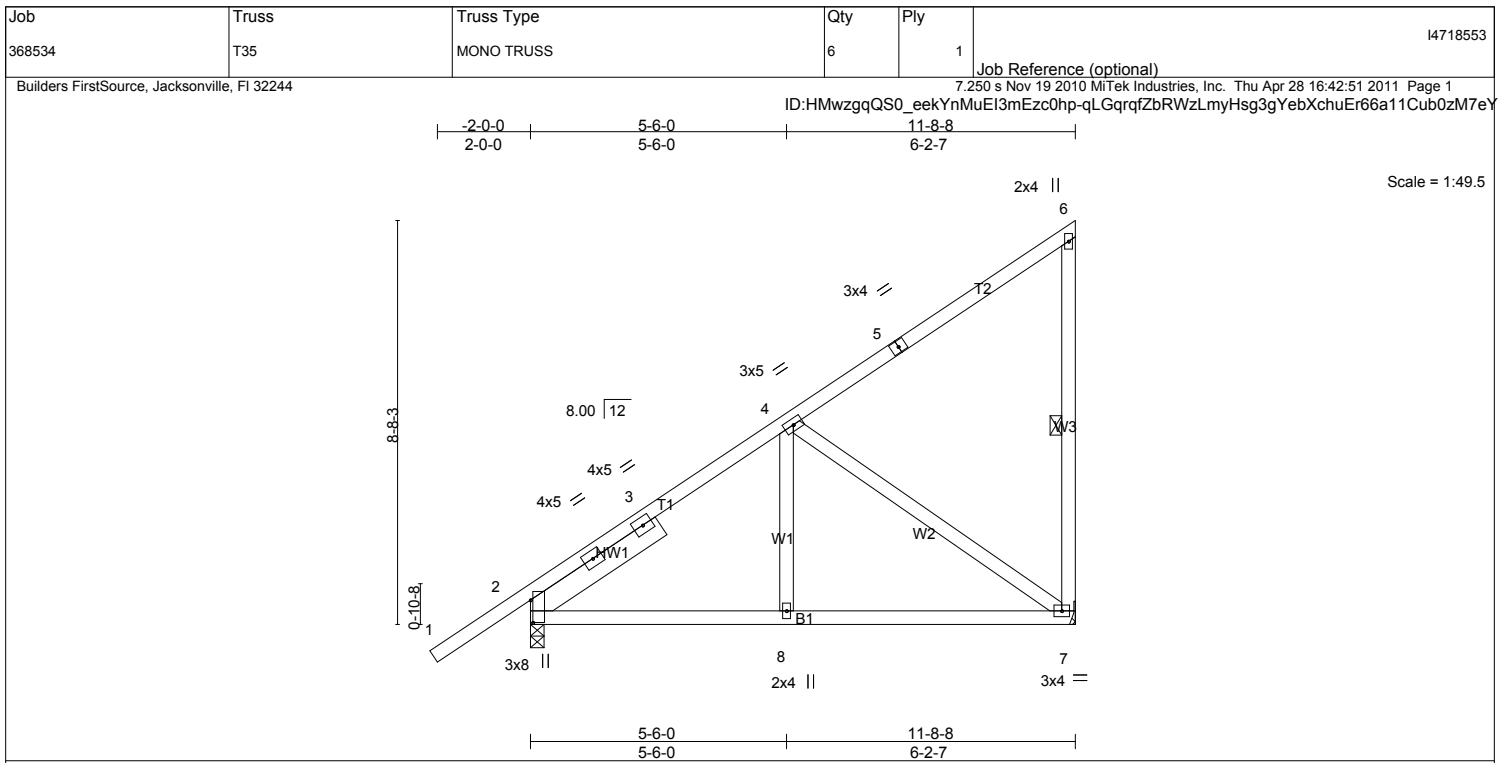


Plate Offsets (X,Y): [2:0-5-13,0-0-10]							
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d
TCLL	20.0	Plates Increase	1.25	TC	0.29	Vert(LL)	-0.03 7-8 >999 240
TCDL	7.0	Lumber Increase	1.25	BC	0.18	Vert(TL)	-0.06 7-8 >999 180
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.32	Horz(TL)	-0.01 7 n/a n/a
BCDL	5.0	Code FBC2007/TPI2002	(Matrix)				
				Weight: 78 lb		FT = 20%	

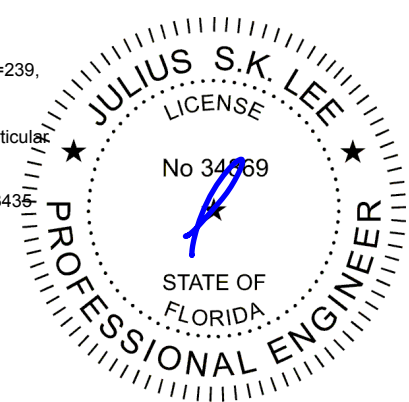
LUMBER		BRACING	
TOP CHORD	2 X 4 SYP No.2	TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD	2 X 4 SYP No.2	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS	2 X 4 SYP No.3	WEBS	1 Row at midpt 6-7
SLIDER	Left 2 X 6 SYP No.2 3-4-3	MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.	

REACTIONS (lb/size) 7=360/Mechanical, 2=492/0-3-8
Max Horz 2=366(LC 6)
Max Uplift 7=-239(LC 6), 2=-144(LC 6)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-446/0, 3-4=-285/0
BOT CHORD 2-8=-314/283, 7-8=-314/283
WEBS 4-7=-336/374

- NOTES** (8-12)
- 1) Wind: ASCE 7-05; 120mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - 4) All bearings are assumed to be SYP No.2 .
 - 5) Refer to girder(s) for truss to truss connections.
 - 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 7=239, 2=144.
 - 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - 8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - 9) For special connections with reactions or uplifts less than 300 lbs. Use typical toe-nail connection (refer to BFS detail package)
 - 10) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435
 - 11) Use Simpson HTU26 to attach Truss to Carrying member

LOAD CASE(S) Standard



April 28, 2011

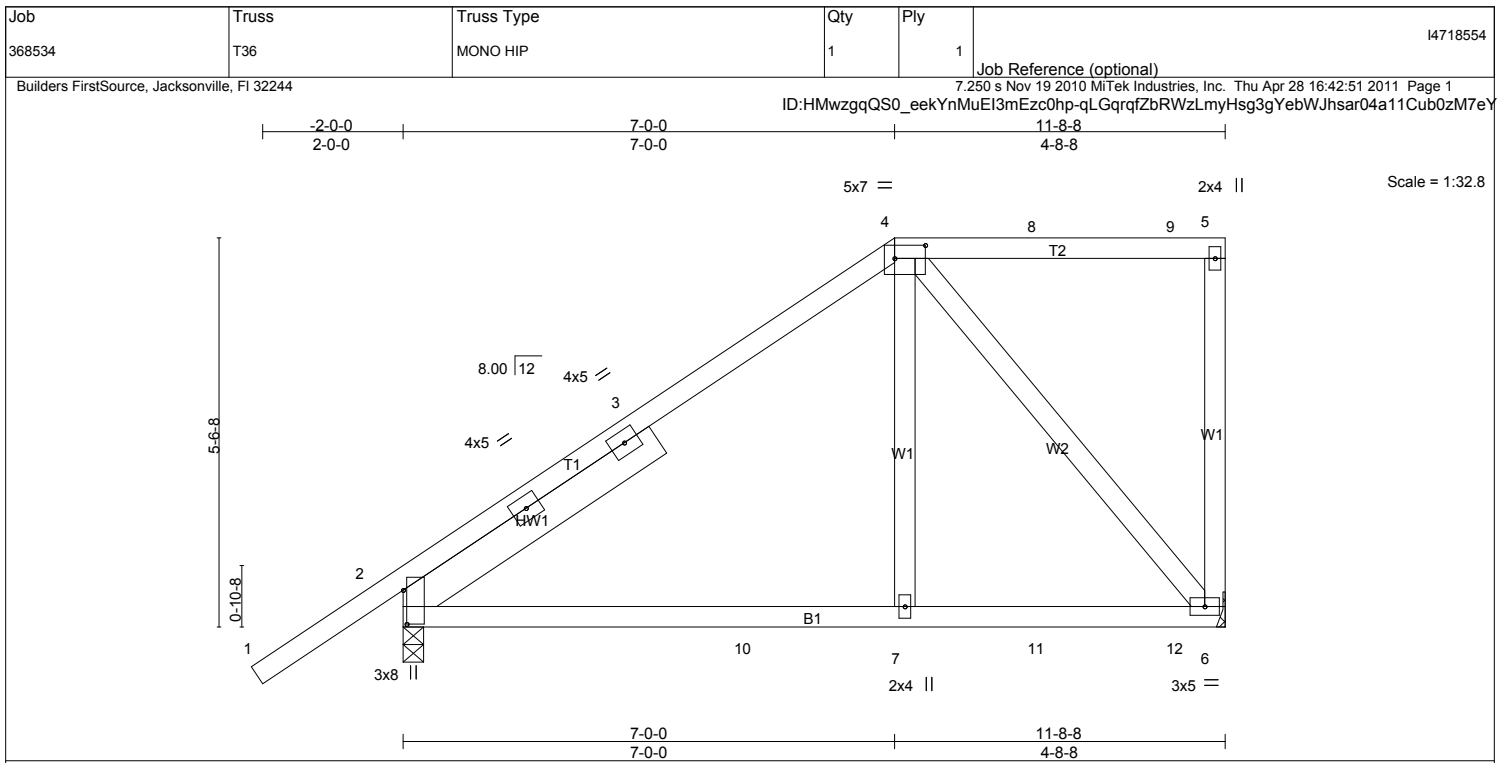


Plate Offsets (X,Y): [2:0-5-13,0-0-10], [4:0-5-4,0-2-4]					
LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.38	in (loc) l/defl L/d	MT20	244/190
TCDL 7.0	Lumber Increase 1.25	BC 0.29	Vert(LL) -0.05 2-7 >999 240		
BCLL 0.0 *	Rep Stress Incr NO	WB 0.64	Vert(TL) -0.09 2-7 >999 180		
BCDL 5.0	Code FBC2007/TPI2002	(Matrix)	Horz(TL) 0.01 6 n/a n/a	Weight: 75 lb	FT = 20%

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD 2 X 4 SYP No.2	BOT CHORD Rigid ceiling directly applied or 9-8-11 oc bracing.
WEBS 2 X 4 SYP No.3	
SLIDER Left 2 X 6 SYP No.2 4-4-0	

REACTIONS (lb/size)	6=907/Mechanical, 2=733/0-3-8
Max Horz 2=250(LC 5)	
Max Uplift 6=662(LC 5), 2=436(LC 5)	

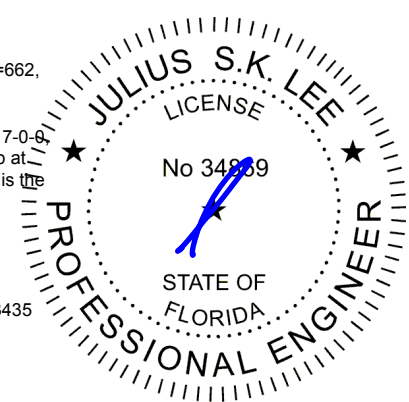
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-775/405, 3-4=-632/423, 5-6=-271/260
BOT CHORD 2-10=-414/525, 7-10=-414/525, 7-11=-419/535, 11-12=-419/535, 6-12=-419/535
WEBS 4-7=-169/531, 4-6=-793/623

- NOTES (11-15)**
- 1) Wind: ASCE 7-05; 120mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise); Lumber DOL=1.60 plate grip DOL=1.60
 - 2) Provide adequate drainage to prevent water ponding.
 - 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 5.0psf.
 - 5) All bearings are assumed to be SYP No.2 .
 - 6) Refer to girder(s) for truss to truss connections.
 - 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 6=662, 2=436.
 - 8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - 9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 207 lb down and 305 lb up at 7-0-8 and 116 lb down and 142 lb up at 9-0-12, and 116 lb down and 142 lb up at 11-0-12 on top chord, and 270 lb down and 220 lb up at 7-0-0, and 74 lb down at 9-0-12, and 74 lb down at 11-0-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
 - 10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
 - 11) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - 12) For special connections with reactions or uplifts less than 300 lbs. Use typical toe-nail connection (refer to BFS detail package)
 - 13) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435
 - 14) Use Simpson HTU26 to attach Truss to Carrying member

LOAD CASE(S)	Standard
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Continued on page 2

April 28, 2011



Job	Truss	Truss Type	Qty	Ply	
368534	T36	MONO HIP	1	1	

I4718554

Job Reference (optional)

Builders FirstSource, Jacksonville, Fl 32244

7.250 s Nov 19 2010 MiTek Industries, Inc. Thu Apr 28 16:42:51 2011 Page 2

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LOAD CASE(S) Standard

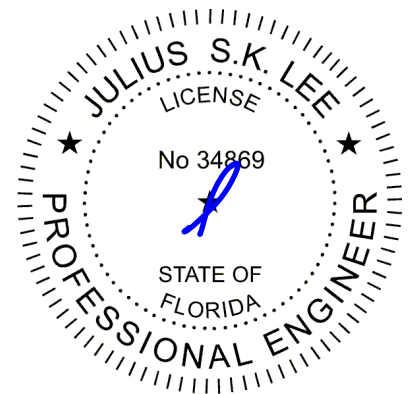
1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-4=-54, 4-5=-54, 2-10=-10, 7-10=-50, 6-7=-10

Concentrated Loads (lb)

Vert: 7=-208(B) 4=-207(B) 8=-116(B) 9=-116(B) 11=-25(B) 12=-25(B)



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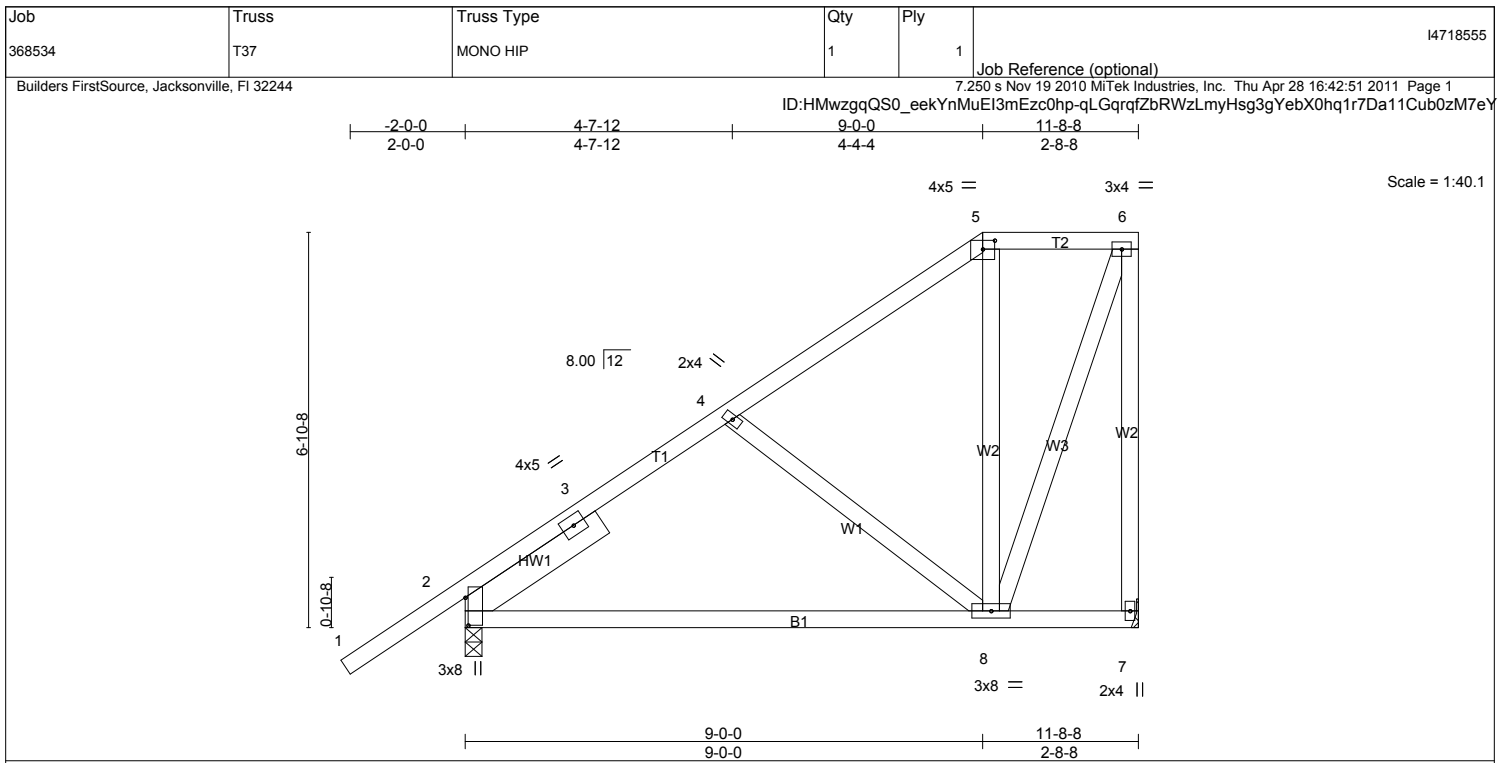


Plate Offsets (X,Y): [2:0-5-13,0-0-10], [5:0-2-8,0-1-13]									
LOADING (psf)		SPACING 2-0-0		CSI		DEFL in (loc) l/defl L/d		PLATES GRIP	
TCLL	20.0	Plates Increase	1.25	TC	0.27	Vert(LL)	-0.14 2-8 >962	240	MT20 244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.39	Vert(TL)	-0.25 2-8 >545	180	
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.25	Horz(TL)	-0.01 7 n/a	n/a	
BCDL	5.0	Code FBC2007/TPI2002		(Matrix)					Weight: 85 lb FT = 20%

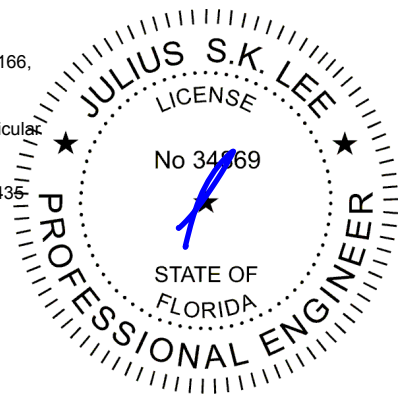
LUMBER		BRACING	
TOP CHORD	2 X 4 SYP No.2	TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD	2 X 4 SYP No.2	BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS	2 X 4 SYP No.3		
SLIDER	Left 2 X 6 SYP No.2 2-10-2		
		MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.	

REACTIONS (lb/size) 7=360/Mechanical, 2=492/0-3-8
Max Horz 2=301(LC 6)
Max Uplift 7=166(LC 6), 2=181(LC 6)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-428/95, 3-4=-293/115, 6-7=-373/332
BOT CHORD 2-8=-326/273
WEBS 4-8=-192/277, 6-8=-294/340

- NOTES** (9-13)
- 1) Wind: ASCE 7-05; 120mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 2) Provide adequate drainage to prevent water ponding.
 - 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - 5) All bearings are assumed to be SYP No.2 .
 - 6) Refer to girder(s) for truss to truss connections.
 - 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 7=166, 2=181.
 - 8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - 10) For special connections with reactions or uplifts less than 300 lbs. Use typical toe-nail connection (refer to BFS detail package)
 - 11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435
 - 12) Use Simpson HTU26 to attach Truss to Carrying member

LOAD CASE(S) Standard



April 28, 2011

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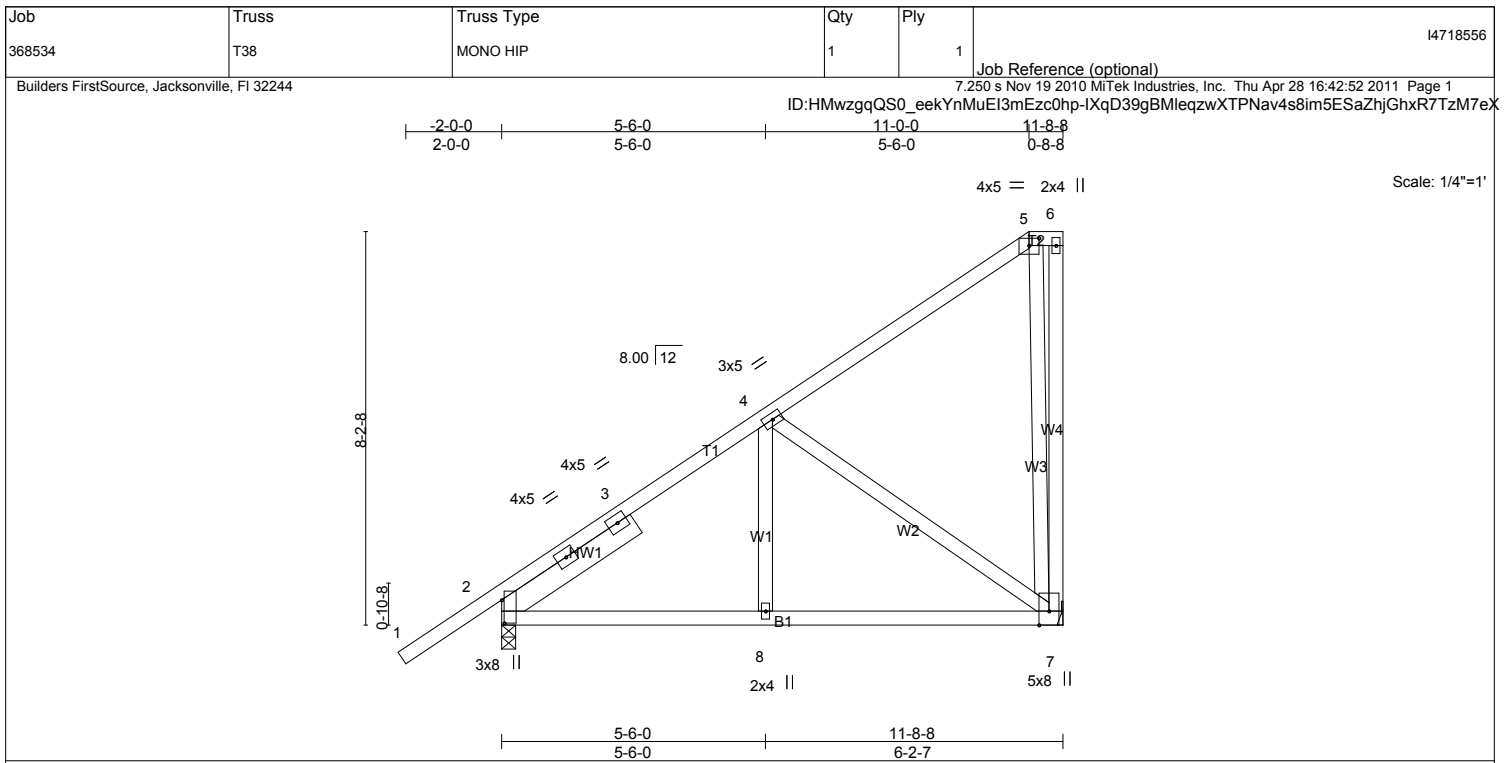


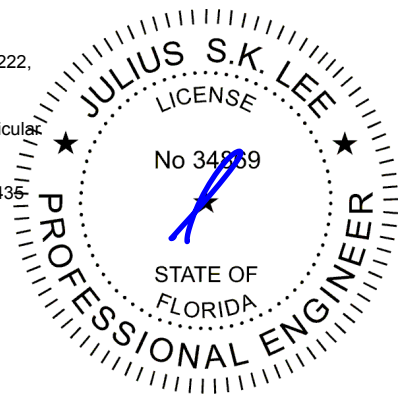
Plate Offsets (X,Y): [2:0-5-13,0-0-10], [5:0-2-8,0-1-13]					
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc) l/defl L/d
TCLL	20.0	Plates Increase	1.25	TC	0.27
TCDL	7.0	Lumber Increase	1.25	BC	0.18
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.30
BCDL	5.0	Code FBC2007/TPI2002	(Matrix)	Horz(TL)	-0.01 7 n/a n/a
			PLATES		GRIP
			MT20		244/190
			Weight: 89 lb		FT = 20%

LUMBER		BRACING	
TOP CHORD	2 X 4 SYP No.2	TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD	2 X 4 SYP No.2	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS	2 X 4 SYP No.3	MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.	
SLIDER	Left 2 X 6 SYP No.2 3-4-3		

REACTIONS (lb/size)		FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.	
7=360/Mechanical, 2=492/0-3-8		TOP CHORD 2-3=-443/0, 3-4=-280/8	
Max Horz 2=352(LC 6)		BOT CHORD 2-8=-304/277, 7-8=-304/277	
Max Uplift 7=-222(LC 6), 2=-153(LC 6)		WEBS 4-7=-314/344, 5-7=-240/300	

- NOTES (9-13)
- 1) Wind: ASCE 7-05; 120mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 2) Provide adequate drainage to prevent water ponding.
 - 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - 5) All bearings are assumed to be SYP No.2 .
 - 6) Refer to girder(s) for truss to truss connections.
 - 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 7=222, 2=153.
 - 8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - 9) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.
 - 10) For special connections with reactions or uplifts less than 300 lbs. Use typical toe-nail connection (refer to BFS detail package)
 - 11) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435
 - 12) Use Simpson HTU26 to attach Truss to Carrying member

LOAD CASE(S) Standard	
April 28, 2011	



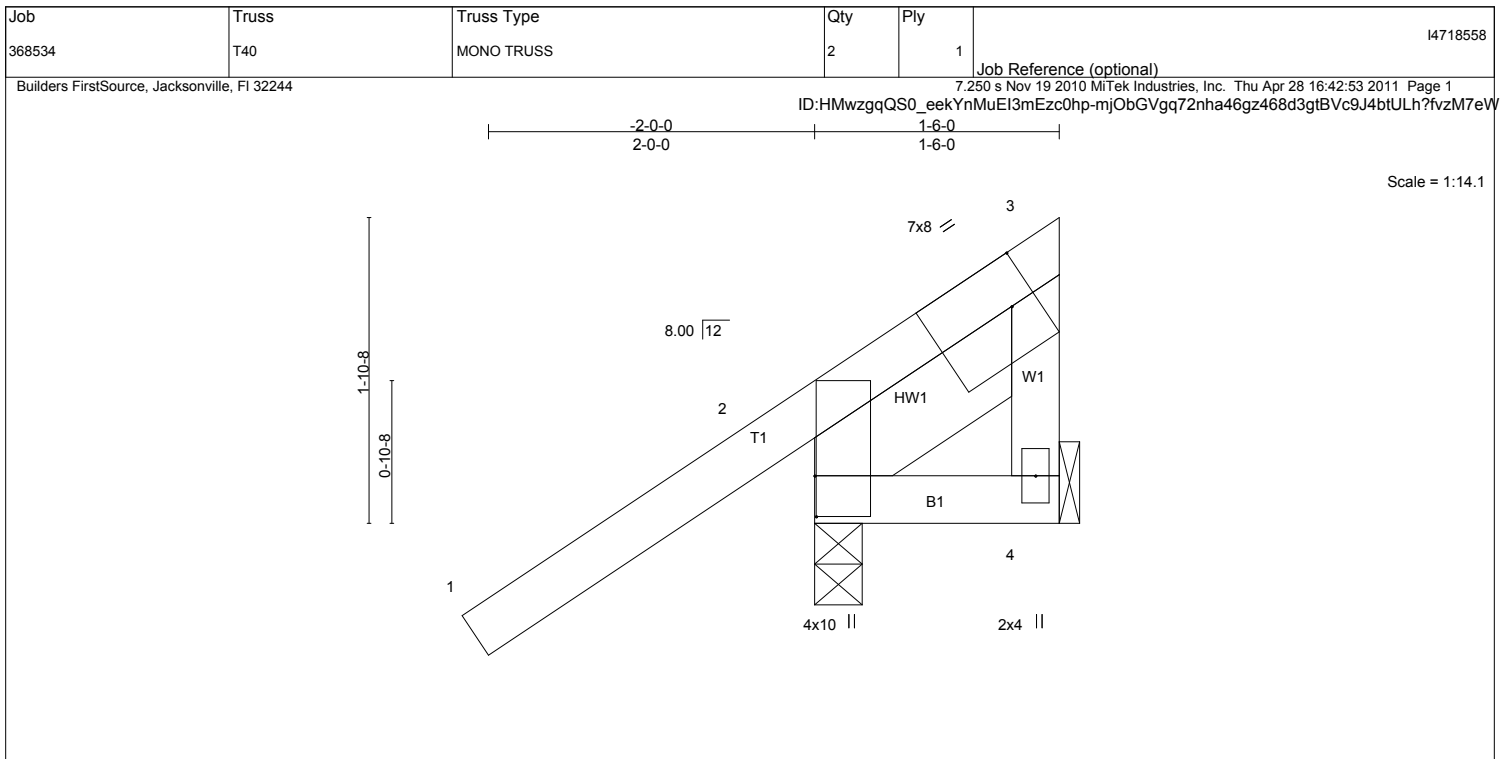


Plate Offsets (X,Y): [2:0-3-0,0-0-2], [3:0-1-14,Edge]											
LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in (loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0		Plates Increase	1.25	TC 0.29		Vert(LL)	-0.00 2	>999	240	MT20	244/190
TCDL 7.0		Lumber Increase	1.25	BC 0.02		Vert(TL)	-0.00 2	>999	180		
BCLL 0.0 *		Rep Stress Incr	YES	WB 0.00		Horz(TL)	0.00	n/a	n/a		
BCDL 5.0		Code FBC2007/TPI2002		(Matrix)						Weight: 14 lb	FT = 20%
LUMBER					BRACING						
TOP CHORD 2 X 4 SYP No.2					TOP CHORD Structural wood sheathing directly applied or 1-6-0 oc purlins, except end verticals.						
BOT CHORD 2 X 4 SYP No.2					BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.						
WEBS 2 X 4 SYP No.3					MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.						
SLIDER Left 2 X 6 SYP No.2 1-7-0											
REACTIONS (lb/size) 2=242/0-3-8, 4=-43/Mechanical											
Max Horz 2=106(LC 6)											
Max Uplift 2=-212(LC 6), 4=-43(LC 1)											
Max Grav 2=242(LC 1), 4=32(LC 6)											
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.											
NOTES (8-11)											
1) Wind: ASCE 7-05; 120mph (3-second gust); TCDL=4.2psf; BCDL=3.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) and C-C Exterior(2) zone; porch left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60											
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.											
3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.											
4) All bearings are assumed to be SYP No.2 .											
5) Refer to girder(s) for truss to truss connections.											
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4 except (jt=lb) 2=212.											
7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.											
8) This manufactured product is designed as an individual building component. The suitability and use of this component for any particular building is the responsibility of the building designer per ANSI TPI 1 as referenced by the building code.											
9) For special connections with reactions or uplifts less than 300 lbs. Use typical toe-nail connection (refer to BFS detail package)											
10) Truss Design Engineer: Julius Lee, PE: Florida P.E. License No. 34869: Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435											
LOAD CASE(S) Standard											

JULIUS S.K. LEE

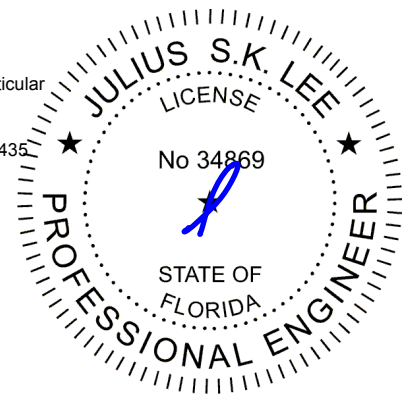
LICENSE

No 34869

PROFESSIONAL ENGINEER

STATE OF FLORIDA

April 28,2011



April 28, 2011

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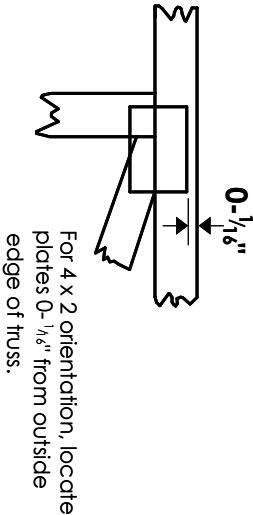
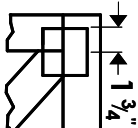
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Boynton, FL 33435

Symbols

PLATE LOCATION AND ORIENTATION

Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and fully embed teeth.



This symbol indicates the required direction of slots in connector plates.

*** Plate location details available in MITek 20/20 software or upon request.**

PLATE SIZE

4 X 4

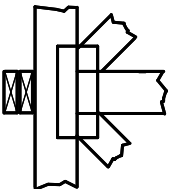
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



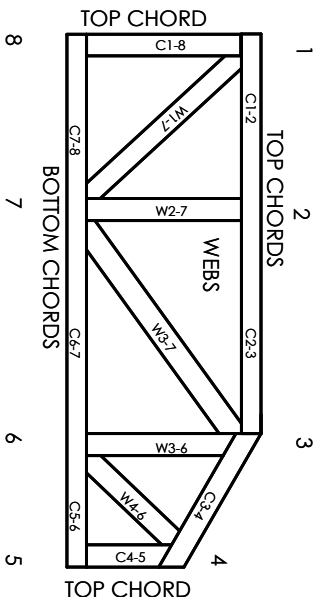
Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

ANSI/FP11: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-89: Design Standard for Bracing.
BCS11: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8 dimensions shown in ft-in-sixteenths (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ER-5243, 9604B, 9730, 95-43, 96-31, 9667A
NER-487, NER-561
95110, 84-32, 96-67, ER-3907, 9432A

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General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCS11.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and ware at joint locations are regulated by ANSI/FP 1.
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/FP 1.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with ANSI/FP 1 Quality Criteria.