DESIGN LOADS: ACTUAL AND UNIFORM

(cd=1.25)TOP CHORD LIVE LOAD 20 psf TOP CHORD DEAD LOAD 20 psf (CLAY TILES) BOTTOM CHORD LIVE LOAD 10 psf

<u>ROOF</u>

BOTTOM CHORD DEAD LOAD 5 psf

FLOOR LOADING TOP CHORD LIVE LOAD (cd=1.00)40 psf TOP CHORD DEAD LOAD 20 psf BOTTOM CHORD LIVE LOAD 0 psf BOTTOM CHORD DEAD LOAD 5 psf

## **DEFLECTION CRITERIA:**

ROOF FRAMING: LIVE LOAD L/240 TOTAL LOAD L/180 FLOOR FRAMING: LIVE LOAD L/480 & TOTAL LOAD L/360

ASCE 7/05 FOR WIND UPLIFT, TRUSSES SHALL BE DESIGNED WITH A MIN. DEAD LOAD CONDITION OF 5 PSF TOP CHORD AND 5 PSF BOTTOM CHORD. REACTIONS CALCULATED FOR THE BEARING POINTS OF ROOF TRUSSES SHALL BE REDUCED. SPECIFICALLY, ATTIC FLOOR LIVE LOADS COMBINED WITH ROOF LIVE LOADS SHALL BE MULTIPLIED BY 0.75 WHEN COMBINED w/ DEAD LOAD.

BASIC WIND SPEED (ASCE 7-10)	130
IMPORTANCE FACTOR	1.00
MEAN ROOF HEIGHT	35.0 FT
ROOF PITCH	VARIES
BUILDING CATEGORY	
EXPOSURE CATEGORY	D
ENCLOSURE CLASSIFICATION	ENCLOSED
INTERNAL PRESSURE COEFFICIENT	± .18

COMPONENTS	& CLADDING DESIGN	PRESSURES
TRIBUTARY AREA (sf)	INTERIOR ZONE (PSF)	EDGE STRIP (PSF): 'a' = 4'-6"
10	+41.6 -45.4	+41.6 -56.0
50	+37.5 -41.0	+37.5 -47.3
100	+35.6 -39.1	+35.6 -43.5

### IHE VALUES ABOVE ARE ALLOWABLE WIND PRESSURE VALUES (ASD). THE ABOVE MIND PRESSURES HAVE BEEN REDUCE BY 0.60 AS PERMITTED BY THE ALLOWABLE STRESS DESIGN METHODOLOGY. NO FURTHER REDUCTION SHALL BE PERMITTED

- COMPONENT & CLADDING WALL ELEMENTS SHALL BE DESIGNED FOR BOTH POSITIVE AND NEGATIVE PRESSURES SHOWN IN TABLE ABOVE.
- LINEAR INTERPOLATION IS PERMISSIBLE.
- PLUS = PRESSURE AND MINUS = SUCTION.
- THE DISTANCE 'a' FROM OUTSIDE CORNERS OF BUILDING SHALL BE DESIGNED FOR EDGE STRIP PRESSURES.
- DESIGN OF WINDOWS/DOORS FASTENING TO THE WALL FRAMING IS THE RESPONSIBILITY OF THE WINDOW/DOOR MANUF./SUPPLIER & SHALL MEET THE ABOVE NOTED POSITIVE AND NEGATIVE PRESSURES.

# GENERAL NOTES

ANCHOR BOLTS & THREADED ROD: SHALL BE IN ACCORDANCE WITH ASTM A 307 OR ASTM F 1554 GRADE 36.

WASHERS: SHALL BE IN ACCORDANCE WITH ASTM A500 (GRADE B).

NUTS: SHALL BE IN ACCORDANCE WITH ASTM A 563 GRADE A HEX. METAL CONNECTORS: ALL METAL CONNECTORS WHICH ARE EXPOSED TO EXTERIOR

RETROFIT REBAR/ROD INSTALLATION: EMBEDMENT OF RODS OR REBAR DOWELS SHALL BE 12 BAR DIAMETER MINIMUM, HOLES SHALL BE  $\frac{1}{4}$ " LARGER THAN REBAR SIX AND

 $\frac{1}{8}$ " LARGER THAN THREADED ROD SIZE. (U.O.N.) ANCHORING ADHESIVE: SHALL BE ONE OF THE FOLLOWING PRODUCTS (DUAL

CARTRIDGE INSTALLATION ONLY): USP CIA-GEL 7000 HIGH STRENGTH EPOXY.

SIMPSON 'SET' EPOXY.

REINFORCING STEEL: SHALL BE ASTM A615, GRADE 60. STRUCTURAL STEEL: SHALL BE ASTM A992, GRADE 50. STRUCTURAL STEEL BOLTS: SHALL BE ASTM A325

SHALL BE NOTIFIED OF ANY DISCREPANCY OR OMISSION.

WELDED WIRE FABRIC (WWF): SHALL BE ASTM A185.

CONTRACTOR RESPONSIBILITIES THE CONTRACTOR SHALL COORDINATE THE ARCHITECTURAL, CIVIL, MECHANICAL, ELECTRICAL AND PLUMBING FOR LOCATION OF MISCELLANEOUS ITEMS (SUCH AS BENT PLATES, INSERTS, ETC.) AFFECTING STRUCTURAL WORK, LOCATION OF OPENINGS AND SLEEVES THROUGH THE STRUCTURE, SLAB DEPRESSIONS, FLOOR DRAINS, INSERTS, AND OTHER RELATED ITEMS WITH THE STRUCTURAL DRAWINGS, THE CONTRACTOR SHALL VERIFY ALL CONTRACT DOCUMENT FLOOR AND ROOF MOUNTED MECHANICAL EQUIPMENT WEIGHTS, FLOOR AND ROOF OPENING SIZES AND LOCATIONS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS AS WELL AS WITH ACTUAL EQUIPMENT PURCHASED. ARCHITECT/STRUCTURAL ENGINEER

COORDINATE THE STRUCTURAL DOCUMENTS WITH THE ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING AND CIVIL DOCUMENTS. ARCHITECT/STRUCTURAL ENGINEER SHALL BE NOTIFIED OF ANY DISCREPANCY OR OMISSION.

NOTIFY IN WRITING THE STRUCTURAL ENGINEER OF CONDITIONS ENCOUNTERED IN THE FIELD CONTRADICTORY TO THOSE SHOWN IN THE STRUCTURAL DOCUMENTS.

CONTRACTOR HAS SOLE RESPONSIBILITY FOR MEANS, METHODS, TECHNIQUES, SEQUENCES, AND PROCEDURES OF CONSTRUCTION.

CONTRACTOR HAS SOLE RESPONSIBILITY FOR THE DESIGN. ADEQUACY. AND SAFETY OF ERECTION BRACING, SHORING, TEMPORARY SUPPORTS.

CONTRACTOR HAS SOLE RESPONSIBILITY TO COMPLY WITH ALL OSHA SAFETY REGULATIONS.

ROOF COVERING SPECIFICATIONS:

CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN AND INSTALLATION OF THE ROOF COVERING SYSTEM. ASPHALT SHINGLES SHALL COMPLY WITH ASTM D3161 AND BE INSTALLED ACCORDING TO THE MANUFACTURER'S REQUIREMENTS

CLAY AND TILE ROOFS SHALL BE INSTALLED PER THE "CONCRETE AND CLAY ROOF TILE INSTALLATION MANUAL." AND THE MANUFACTURER'S REQUIREMENTS. STANDING SEAM METAL ROOFS SHALL COMPLY WITH ASTM E1514 AND BE INSTALLED ACCORDING TO THE MANUFACTURER'S REQUIREMENTS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN AND INSTALLATION OF ALL METAL FLASHING AND VALLEY MATERIALS.

PRESSURE TREATED WOOD FRAMING SPECIFICATIONS:

ALL WOOD MEMBERS EXPOSED TO WEATHER OR IN CONTACT WITH MASONRY, CONCRETE OR SOIL SHALL BE PRESSURE-TREATED. PT LUMBER INDICATED IN SECTIONS PRESERVED WITH SODIUM BORATE (DOT-DISODIUM OCTABORATE TETRA HYDRATE), IF HOWEVER, ACQ OR NON-DOT BORATE PRESERVATIVE TREATMENT IS USED, ALL ATTACHED FASTENERS SHALL BE HOT DIPPED GALVANIZED. IF ACZA PRESERVATIVE IS USED, ALL ATTACHED FASTENERS SHALL BE STAINLESS STEEL.

TIMBER FRAMED BEARING WALL NOTES:

. DOUBLE TOP PLATES AND SILL PLATES SHALL BE SPF#2. 2. PLATES IN CONTACT WITH CONCRETE OR CMU SHALL BE P.T. (SEE ABOVE)

3. SILL PLATE ANCHORS -\( \mathcal{H}^{\infty} \) x6" EPOXY ANCHORS @ 48" O.C. MAX 4. STUD SIZE & GRADE PER STRUCTURAL PLANS.

5. MIN. OF SPF UTILITY GRADE FOR INTERIOR NON-BRG. TOP PLATES. 6. USE SPF#2 OR BETTER FOR ALL WALLS IN THESE PLANS, U.O.N.

7. SPACE STUDS AT 24" O.C. IN NON-LOAD BEARING WALLS. 8. ALL WALLS SHALL BE <u>BALLOON</u> FRAMED FULL HEIGHT TO ROOF OR FLOOR BEARING ELEVATION, UNLESS NOTED OTHERWISE ON PLAN.

### GALVANIZED METAL STRAPS AND FASTENERS

ALL METAL STRAPS AND HANGERS SHALL BE GALVANIZED G185 (1.85 oz/FT^2) OR BETTER. SIMPSON PRODUCTS SHALL BE Zmax GALVINZED.

HANGERS AND STRAPS INTALLED OVER ACQ OR ACZE PRESSURE TREATED LUMBER SHALL BE STAINLESS STEEL.

ALL WOOD FASTENERS (NAILS, SCEWS, BOLTS, ETC.) SHALL BE HOT DIPPED GALVANIZED TO MEET THE SPECIFICATIONS OF ASTM153 OR BETTER.

FASTENERS INTALLED INTO ACQ OR ACZE PRESSURE TREATED LUMBER SHALL BE STAINLESS STEEL.

ROOF - MIN.  $1\frac{1}{3}$ , 24/16, APA RATED OSB OR PLYWOOD SHEATHING, NAILED w/ 0.113x2\(\frac{1}{2}\)" RING SHANK NAILS @ 6" O.C. EDGE & 6" O.C. FIELD (AT GABLE ENDS DECREASE EDGE NAIL SPACING TO 4" O.C. WITHIN 4'-0" OF ROOF EDGE).

WALL - MIN.  $\frac{7}{16}$ ", 24/16, APA RATED OSB **OR** PLYWOOD SHEATHING, FASTENED w/ 8d @ 6" O.C. EDGE AND 6" O.C. FIELD. SEE PLAN FOR INCREASED NAILING REQUIREMENTS AT SHEARWALLS

FLOOR - 23/32" T&G OSB OR PLYWOOD SHEATHING, GLUE AND NAIL WITH 10d COMMON @ 6" O.C. EDGE & FIELD

MASONRY HAS BEEN DESIGNED IN ACCORDANCE WITH ACI 530-05, AND SHALL BE CONSTRUCTED IN ACCORDANCE WITH ACI530.1-05 GROUT SHALL BE IN ACCORDANCE WITH ASTM C476 WITH A MINIMUM OF 28 DAY COMPRESSIVE STRENGTH OF 2500 psi PER ASTM C1019, GROUT SHALL HAVE A MAXIMUM COURSE AGGREGATE SIZE OF  $\frac{3}{8}$  PLACED AT AN 8" TO 11" SLUMP. MORTAR SHALL CONFORM TO ASTM C270 AND TYPE M OR S. TYPE N MORTAR MAY BE USED IN BRICK VENEER. CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN AND INSTALLATION OF ALL FLASHING. CMU SHALL BE IN ACCORDANCE WITH ASTM C90-75, HOLLOW LOAD-BEARING (CMU), TYPE 1, GRADE N-1, NORMAL WEIGHT, WITH A MINIMUM COMPRESSIVE STRENGTH OF 1900 psi (f'm=1500 psi). GROUT ALL CELLS CONTAINING VERTICAL REINFORCEMENT IN 5'-0" MAXIMUM LIFTS PROVIDE CLEANOUTS PER ACI 530.1-02 IN THE BOTTOM OF COURSE OF MASONRY WHEN THE WALL HEIGHT EXCEEDS 5'-0".

MASONRY STEMWALLS: ALL CONCRETE MASONRY UNITS SHALL BE COMPOSED OF ASTM C90E, E GRADE N-1 HOLLOW CONCRETE MASONRY UNITS WITH TYPE 'S' MORTAR. WALL COURSING SHALL BE RUNNING BONDS, STACK BOND SHALL NOT BE USED. GROUT ALL CELLS CONTAINING VERTICAL REINFORCEMENT WITH 4000 PSI PEA ROCK CONCRETE GROUT. SPLICES IN REINFORCING, WHERE PERMITTED, SHALL BE 48 BAR DIAMETERS ALL CELLS BELOW FINISHED GRADE SHALL BE GROUTED SOLID. ALL EXTERIOR WALLS SHALL BE REINFORCED FULL HEIGHT WITH - #4 @ 4'-0" O.C. MAX. AND AT EACH CORNER, WALL END, AND WALL INTERSECTIONS. PROVIDE CONTINUITY OF REINFORCING AT INTERSECTIONS OF PERPENDICULAR MASONRY ELEMENTS BY INSTALLING CORNER BARS, MINIMUM OF 40 BAR DIAMETERS INTO EACH ELEMENT. AT STEMWALL CONSTRUCTED OF 5 OR MORE COURSES, PROVIDE HORIZONTAL JOINT REINFORCEMENT AT 16" O.C. VERTICALLY, (EVERY OTHER COURSE). LAP JOINT REINFORCING SHALL BE A MINIMUM OF

CLAY MASONRY (BRICK)

BRICK SHALL BE IN ACCORDANCE WITH ASTM C62, C216, OR C652 FOR BUILDING BRICK, FACING BRICK, & HOLLOW BRICK, RESPECTFULLY.

**CONCRETE SPECIFICATIONS:** 

ALL CONCRETE HAS BEEN DESIGNED IN ACCORDANCE WITH ACI 318-08, AND SHALL BE CONSTRUCTED IN ACCORDANCE WITH ACI 301. ALL CONCRETE SHALL HAVE A COMPRESSIVE STRENGTH OF 3000 PSI AT 28 DAYS

## REINFORCED CONCRETE

COMPRESSIVE STRENGTH REQUIRED, U.O.N.

f'c=5000 PSI AUGERCAST PILES f'c=4000 PSI PILE CAPS & GRADE BEAMS f'c=4000 PSI SLAB-ON-GRADE CONCRETE BEAMS & COLUMNS f'c=4000 PSI

ALL CONCRETE WORK SHALL CONFORM TO THE REQUIREMENTS OF THE LATEST EDITION OF ACI 318 AND ACI 301. CONCRETE MIX DESIGNS SHALL BE SUBMITTED FOR THE REVIEW AND APPROVAL OF THE STRUCTURAL ENGINEER.

UNLESS OTHERWISE SHOWN ON DRAWINGS, MINIMUM COVER FOR REINFORCING SHALL BE AS FOLLOWS:

GRADE BEAM (TO MAIN REINF) COLUMNS AND PEDESTALS (TO MAIN REINF) SLABS ON GRADE

ALL CONCRETE WORK SHALL CONFORM TO THE REQUIREMENTS OF THE LATEST EDITION OF ACI 318 AND ACI 301. CONCRETE MIX DESIGNS SHALL BE SUBMITTED FOR THE REVIEW AND APPROVAL OF THE STRUCTURAL ENGINEER.

UNLESS OTHERWISE SHOWN ON DRAWINGS, MINIMUM COVER FOR REINFORCING SHALL BE AS FOLLOWS:

ALL REINFORCEMENT SHALL CONFORM TO ASTM A615, GRADE 60, AND BE FREE FROM LOOSE RUST AND SCALE. ALL WELDED WIRE FABRIC SHALL CONFORM TO ASTM A185.

ALL HOOKS IN REINFORCING BARS SHALL BE ACI STANDARD HOOKS.

PROVIDE 3/4" CHAMFER AT ALL EXPOSED CORNERS OF POURED-IN-PLACE CONCRETE.

ALL CONCRETE REINFORCEMENT SHALL BE DETAILED, FABRICATED, LABELED, SUPPORTED AND SPACED IN FORMS AND SECURED IN PLACE IN ACCORDANCE WITH THE PROCEDURES AND REQUIREMENTS OUTLINED IN THE LATEST EDITION OF THE 'MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES', ACI 315.

BAR SUPPORTS SHALL BE PLASTIC DIPPED AFTER FABRICATION.

SLAB BOLSTERS SHALL BE PROVIDED FOR VERTICAL COLUMN REINFORCING STEEL, SUCH THAT 2 INCH MINIMUM CLEARANCE IS MAINTAINED.

ALL REINFORCING SPLICES SHALL CONFORM TO THE REQUIREMENTS OF THE LATEST EDITION OF ACI 318, BUT IN NO CASE SHALL BE LESS THAN 40 BAR DIAMETERS, U.O.N.

ALL WELDED WIRE FABRIC SHALL BE LAPPED TWO (2) MESH PANELS AND TIED SECURELY. WHERE REQUIRED, DOWELS SHALL MATCH SIZE AND NUMBER OF MAIN REINFORCING, U.O.N.

SUPPORT BARS SHALL BE #5 OR GREATER, AND NOT SPACED MORE THAN 48" O.C. A MINIMUM OF THREE (3) SUPPORT BARS AND THREE (3) INDIVIDUAL CHAIRS FOR EACH SUPPORT BAR SHALL BE PROVIDED FOR TOP BARS.

SUPPORT BARS AND ENDS OF MAIN REINFORCING SHALL NOT EXTEND MORE THAN 1'-6" PAST OUTERMOST CHAIR OR SUPPORT BAR.

FOOTING AND FOUNDATIONS: FOOTINGS AND FOUNDATIONS SHALL BE IN ACCORDANCE WITH THE GEOTECHNICAL INVESTIGATION REPORT PREPARED BY LEGACY ENGINEERING INC, DATED JULY 16, 2012 PROJECT NO 12-1067.1 THE CONTRACTOR SHALL PREPARE THE FOUNDATION SITE PER GEOTECHNICAL REPORT.

IT IS THE RESPONSIBILITY OF THE CONTRACTOR/OWNER TO HIRE A GEOTECHNICAL ENGINEER TO VERIFY PILE DEPTH. THE PILE DEPTH MAY BE MODIFIED AFTER A GEOTECHNICAL INVESTIGATION IS PERFORMED.

FOUNDATION PLAN ONLY CONVEYS STRUCTURAL INFORMATION. FOR GENERAL FEATURES, CONDUITS, ELECTRICAL EMBEDS, STEP HEIGHTS, ET., SEE ARCHITECTURAL PLANS.DO NOT SCALE FOOTING DIMENSIONS AND LOCATION FROM THE FOUNDATION PLAN SHOWN ON S1.0. DO NOT DETERMINE FOOTING LOCATION BASED ON EITHER THE ARCHITECTURAL PLAN OR FRAMING PLAN, BUT BY DIMENSIONS PROVIDED ON FOUNDATION PLAN. IF FOOTING SIZE OR LOCATION IS NOT DETERMINED ON PLAN THEN CONTACT ENGINEER OF RECORD (EOR)

**CONCRETE SLABS ON GRADE:** SHALL BE INSTALLED OVER MINIMUM 6 MIL POLYETHYLENE VAPOR RETARDER WITH JOINTS LAPPED 6" AND SEALED OVER CLEAN, COMPACTED EARTH OR FILL WITH APPROVED CHEMICAL SOIL TREATMENT FOR PREVENTION OF SUBTERRANEAN TERMITES.

SAWCUTS (FOR SLABS ON GRADE WITHOUT REBAR): FOR CONTROLLED CRACKING CUT A 1" SAWCUT INTO SLAB IN A 20'x20' GRID WITHIN 12 HOURS OF CONCRETE PLACEMENT, PROVIDE SAWCUTS THROUGH OUT SLAB AND WHERE SHOWN ON FOUNDATION PLAN S1.01. CALL EOR FOR ALTERNATIVE METHODS.

DESIGN OF PRE-ENGINEERED SYSTEMS SPECIFIED IN THE CONTRACT DOCUMENTS WHICH ARE DESIGNED/ENGINEERED BY OTHERS IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR. SUBMITTALS OF SUCH SYSTEMS SHALL BE SEALED BY AN ENGINEER LICENSED IN THE PROJECT STATE. REVIEW OF SUBMITTALS BY THE STRUCTURAL

ENGINEER SHALL BE FOR CONFORMANCE WITH CONTRACT DOCUMENTS WITH REGARD TO THE ARRANGEMENT AND SIZES OF MEMBERS SHOWN ON THE STRUCTURAL CONTRACT DOCUMENTS AND THE CONTRACTORS INTERPRETATION OF THE DESIGN INFORMATION INCLUDED IN THE CONTRACT DOCUMENTS. SUCH REVIEW BY THE STRUCTURAL SHALL NOT IMPLY ANY RESPONSIBILITY FOR THE ACTUAL DESIGN OF SUCH SYSTEMS. CONTRACTOR HAS FULL RESPONSIBILITY FOR DIMENSIONAL ACCURACY AND CONFORMANCE WITH THE INFORMATION CONTAINED IN THE CONTRACT DOCUMENTS.

PRE-ENGINEERED SYSTEMS SHALL INCLUDE WOOD FLOOR & ROOF SYSTEMS. ALUMINUM & GLASS WINDOWS, DOORS, SEE GENERAL NOTES FOR DESIGN CRITERIA AND ADDITIONAL REQUIREMENTS. FOOTINGS. HOLDDOWNS. FASTENERS. ETC. INDICATED WITHIN WHICH SUPPORT A PRE-ENGINEERED ITEM ARE PRELIMINARY AND SUBJECT TO CHANGE AFTER REVIEW OF THE PRE-ENGINEERED SHOP DRAWINGS. THESE SHOP DRAWINGS SHALL ALSO BE SUBMITTED TO THE BUILDING DEPT. AS A DEFERRED SUBMITTAL TO THE PERMIT DOCUMENTS.

## SHOP DRAWINGS:

ALL REINFORCING AND STRUCTURAL STEEL SHOP DRAWING ONE PAPER (SEPIA AND ONE PRINT ) SHALL BE SUBMITTED TO THE ARCHITECT/ ENGINEER FOR APPROVAL.

IN ACCORDANCE WITH CHAPTER 21-H, FLORIDA ADMINISTRATIVE CODE, THE FOLLOWING WRITTEN REQUIREMENTS ARE TO BE FOLLOWED BY THE DELEGATED ENGINEER IN PREPARING HIS ENGINEERING DOCUMENTS ON THAT PORTION OF THE PROJECT THAT UTILIZES PREFABRICATED COMPONENTS.

- A. ALL SHOP DRAWINGS SHALL ADHERE TO THE ARCHITECT OR ENGINEER OF RECORD FRAMING PLANS UNLESS PRIOR WRITTEN APPROVAL IS OBTAINED FROM THE ARCHITECT OR ENGINEER OF RECORD. IF RE-FRAMING IS APPROVED, THE ARCHITECT OR ENGINEER OF RECORD SHALL RESUBMIT REVISED FRAMING PLANS TO THE BUILDING OFFICIAL AFTER RECEIVING UPDATED PLANS FROM THE DELEGATED ENGINEER SHOWING ALL ADJUSTMENTS NECESSARY TO SAFELY TRANSMIT ALL APPLIED LOADS TO THE FOUNDATION
- B. THE DELEGATED ENGINEERING DOCUMENTS SHALL SHOW ALL PERMANENT BRACING AS DEFINED ON THE ARCHITECT OR ENGINEER OF RECORD PLANS ALONG WITH INDIVIDUAL BRACING REQUIRED TO SECURE THE ENTIRE SYSTEM INCLUDING CONNECTIONS TO THE LOAD RESISTING STRUCTURAL SYSTEM., UNDER ALL DESIGN CONDITIONS NOTED ON THE ARCHITECT OR ENGINEER PLANS.
- C. ALL DETAILS AND SECTIONS REQUIRED TO SHOW THE SIZE AND CONNECTIONS OF ALL SECONDARY MEMBERS WILL BE SUPPLIED ON THE DELEGATED ENGINEERING PLANS AND SHALL SHOW ALL FRAMING, CONNECTIONS AND BRACING ON ONE OR MORE PRIMARY PLANS OF MINIMUM SIZE 24" X 36".

SHOP DRAWING SUBMITTAL INDEX PRE-ENGINEERED WOOD TRUSSES CONCRETE BEAM & COLUMN REBAR PLACEMENT MASONRY WALL REBAR PLACEMENT CONCRETE MIX DESIGNS

	SIMPS	ON C	ONNECTORS	
	UF	PLIFT		
CONNECTOR	SYP	SPF	FASTENERS	FL# CODE
A35	450	450	12-8dx1 <sup>1</sup> / <sub>2</sub> "	10446.4
H2.5T	600	520	5-8d EA. END	11478.3
H8-1/2	620	530	$5-10dx1\frac{1}{2}$ " EA. END	11470.3
MTS12	1000	860	$7-10d \times 1^{1/2}$ " EA. END	10456.3
HTS20	1450	1245	$24-10dx1\frac{1}{2}$ " EA. END	13872.3
MSTA24	1765	1270	9-10d EA. END	13872.4
MSTA36	2050	1870	13-10d EA. END	13872.8
MSTAM24	1465	1270	9-10d EA. END	11473.19
MUSTAMET	1403	1270	5-1/4"x2-1/4 TITENS	11473.13
LUS28	930	780	6-10d TO HEADER	10655.113
LU320	930	780	4-10d TO JOIST	10655.115
1111440	005	705	14-16d TO HEADER	105.71.70
HU410	905	785	6-16d TO JOIST	10531.36
SET	N/A	N/A	SIMPSON EPOXY-TIE	11506.4
LSTA12	805	695	10-10d	13872.5
CS16	1705	1705	13-8d	10852.1

LOCATION	CONNECTION	FASTENER
CEILING JOIST LAPS OVER	(3)16d	54.05
PARTITIONS	(4)GUN NAIL	FACE NAIL
	(3)10d	
COLLAR TIE TO RAFTER	(3)GUN NAIL	FACE NAIL
	(3)8d	
RAFTER TO PLATE	(3)GUN NAIL	TOENAIL
	(3)10d	
	(4)GUN NAIL	TOENAIL
JACK RAFTER TO HIP	(2)16d	
	(3)GUN NAIL	FACE NAIL
ROOF RAFTER TO (2) PLY	(2)16d	TOENAIL OR
RIDGE BEAM	(3)GUN NAIL	FACE NAIL
	(3)16d	
JOIST TO BAND JOIST	(4)GUN NAIL	FACE NAIL
BLOCKING BETWEEN JOISTS OR	(3)8d	
RAFTERS TO TOP PLATE	(3)GUN NAIL	END NAIL
RIM JOIST TO TOP PLATE	8d @ 6"O.C.	TOENAIL
THIN USIST TO TOLL TEXTE	(3)GUN NAIL @ 6"O.C.	TOLIVAIL
TOP PLATES, LAPS AND INTERSECTIONS	(2)16d	FACE NAIL
TIVIENOLO HONO	(3)GUN NAIL @ 6"O.C.	
CEILING JOISTS TO PLATE	(3)8d	TOENAIL
	(5)GUN NAIL	
JOIST TO SILL OR HEADER	(3)8d	TOE NAIL
0" 0110 51 000 70	(2)GUN NAILS	DI IND. OD
2" SUB FLOOR TO JOIST OR GIRDER	(2)16d	BLIND OR FACE NAIL
SOLE PLATE TO JOIST OR BLOCKING	16d @ 16"O.C. (3)GUN NAIL @ 8"O.C.	TYPICAL FACE
	(2)16d	
TOP PLATE TO STUD	(3)GUN NAIL	END NAIL
	(4)8d	
	(4)GUN NAIL	TOENAIL
STUD TO SOLE PLATE		
	(2)16d	END NAIL
	(3)GUN NAIL TYPICAL FASTENERS	
$3" \times 0.131" \phi = GUN NAILS$ $2" \times 0.113" \phi = 6d$ $3" \times 0.148" \phi = 10d$	TYPICAL FASTENERS $2" \times 0.113" \phi = RINK SHANK$ $2^{1}/_{2}" \times 0.131" \phi = 8d$ $3^{1}/_{2}" \times 0.162" \phi = 16d$	$\frac{1}{2}$ "x0.131" $\phi$ = 8dx1 $\frac{1}{2}$ " $\frac{1}{2}$ "x0.148" $\phi$ = 10dx1 $\frac{1}{2}$

	TRUCTURAL DRAWING INDEX
S0.0	DESIGN CRITERIA AND GENERAL NOTES
S1.0	PILE FOUNDATION PLAN
S1.01	OFF-GRADE FRAMING PLAN
S1.02	PILE & PIER DETAILS
S1.03	PILE CAP & CONCRETE BEAM DETAILS
S1.1	FIRST LEVEL WALL FRAMING PLAN
S1.2	FIRST LEVEL FLOOR & ROOF FRAMING PLAN
S2.0	SECOND LEVEL WALL FRAMING PLAN
S2.1	SECOND LEVEL FLOOR FRAMING PLAN
S3.0	THIRD LEVEL WALL FRAMING PLAN
S3.1	THIRD LEVEL ROOF FRAMING PLAN
S4.0	MASONRY DETAILS
S4.1	FRAMING & TIE-DOWN DETAILS
S5.0	SCHEMATIC ELEVATION
S5.1	SCHEMATIC ELEVATION

Lou Pontigo and Associates, Inc. 420 Osceola Avenue Jax. Beach, Florida 32250 Ph. 242-0908 Fax. 241-9557 FL: CA # 8344 SC: CA# 3579 COMMENTS @ LP-A.COM

REVISIONS

FIFLD ALTERATION NTRACTOR SHALL CONTACT L

FROM THE INTENT OF THE ORIGINA LD ALTERNATIONS MADE PRIOR EING APPROVED BY LOU PONTIG ASSOCIATES MAY RESULT IN ADDITIONAL ENGINEERING OF INSPECTION FEES.

MAKING ANY STRUCTURAL FIELD

**DESIGN CRITERIA GENERAL** 

**NOTES** UNCLEAR REFER TO THE RCHITECTURAL DRAWINGS
CONTACT THE E.O.R.

KANE RESIDENCE DESIGN/DRAWN/CHECKED CS / CS / LAP 01-11-2013 CONTROL NO. KANE TRUSS ID.

> SHEET SHEET 1 OF 15

425247 LPA NO.

DRBD-12-00299



<u>DEP CERTIFICATION</u>
THESE PLANS ARE IN CONFORMANCE WITH STANDARDS ESTABLISHED IN SECTION 62B-33.007 FAC

FEMA RELATED DESIGN CRITERIA

FEMA (BFE) - FEMA VE 13 (NGVD) + 1FT SJC = 14 (NGVD) = +12.9 NAVD

DEP MÖNUMENT NUMBER — RO74 100 YR. STORM ELEVATION - +16.1 NAVD (FROM SITE SPECIFIC INVESTIGATION)

100 YR. STORM DESIGN GRADE ELEVATION - +4.4 NAVD (FROM MONUMENT)

BOTTOM OF PILING - -12.0 NAVD BOTTOM OF LOWEST HORIZONTAL STRUCTURAL ELEVATION - +16.1 NAVD

VE CONSTRUCTION CERTIFICATION STATEMENTS

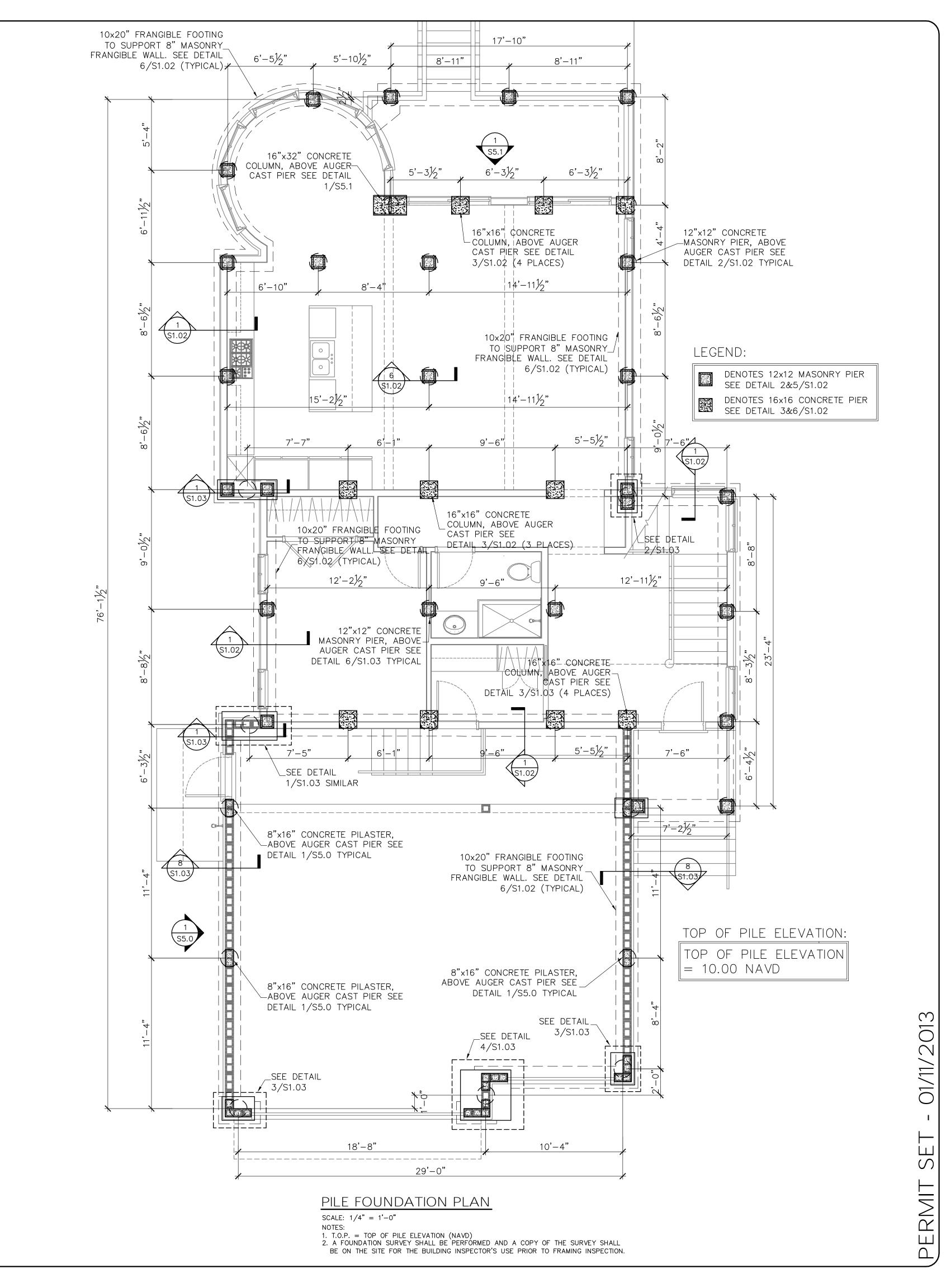
AT THE FEW LOCATIONS WHERE THE PILE SPACING HAS BEEN REDUCED TO LESS THAN 8:1 RATIO (PER FBC 3109.4), THE PILES HAVE BEEN DESIGNED TO RESIST THE ADDITIONAL LOADS FROM FLOOD WATERS AND TRAPPÉD DEBRIS.

FOUNDATION AND STRUCTURE IS ANCHORED TO RESIST FLOTATION, COLLAPSE AND LATERAL MOVEMENT DUE TO THE COMBINED EFFECTS OF WIND AND WATER LOADS ACTING SIMULTANEOUSLY ON THE BUILDING COMPONENTS THAT RECEIVE SUCH LOADING. BREAKAWAY ELEMENTS COMPLY WITH THE INTENT OF TECHNICAL BULLETIN 5 'FREE OF OBSTRUCTION REQUIREMENTS'.

AUGER CAST PILE FOUNDATION

1. FOUNDATION IS SUPPORTED BY 16" DIAMETER AUGER CAST PILES WITH A GROUT STRENGTH OF 5000 PSI AND IS ASSUMED TO HAVE A CAPACITY OF 40 TONS IN COMPRESSION, 10 TONS IN TENSION AND 4 KIPS IN SHEAR.

- CONCRETE COVER OF 2" TO THE #3 2. ALL AUGURED PILE REINFORCEMENT SHALL HAVE A MINIMUM
- 3. AUGURED PILE PLACEMENT TOLERANCE: PLUS OR MINUS 2 INCHES.
- 4. THE CONTRACTOR SHALL ENGAGE THE SERVICE OF A FLORIDA REGISTERED PROFESSIONAL ENGINEER OR AN APPROVED TESTING AGENCY TO SUPERVISE AND CERTIFY THAT THE PILE INSTALLATION COMPLIES WITH THE DESIGN REQUIREMENTS. RECORDS OF THE REINFORCING FOR EACH PILE; EACH INDIVIDUAL PILE DEPTH QUANTITY OF CONCRETE GROUT VOLUME INSTALLED FOR EACH PILE; TOTAL NUMBER OF INSTALLED PILES; TOTAL LENGTH OF INSTALLED PILES; TOTAL CALCULATED CONCRETE GROUT VOLUME FOR ALL OF THE PILES; ACTUAL TOTAL CONCRETE GROUT VOLUME INSTALLED FOR ALL OF THE PILES; AND DEVIATION OF EACH PILE FROM THE SPECIFIED LOCATION SHALL BE KEPT AND SUBMITTED TO THE ENGINEER OF RECORD UPON COMPLETION OF THE AUGURED PILE WORK.



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 $\Box$ 

PILE

**FOUNDATION** 

PLAN

NOT SCALE DIMENSIONS FROM

PLAN NAME

KANE RESIDENCE

DESIGN/DRAWN/CHECKED

01-11-2013 CONTROL NO.

> KANE TRUSS ID.

425247

LPA NO.

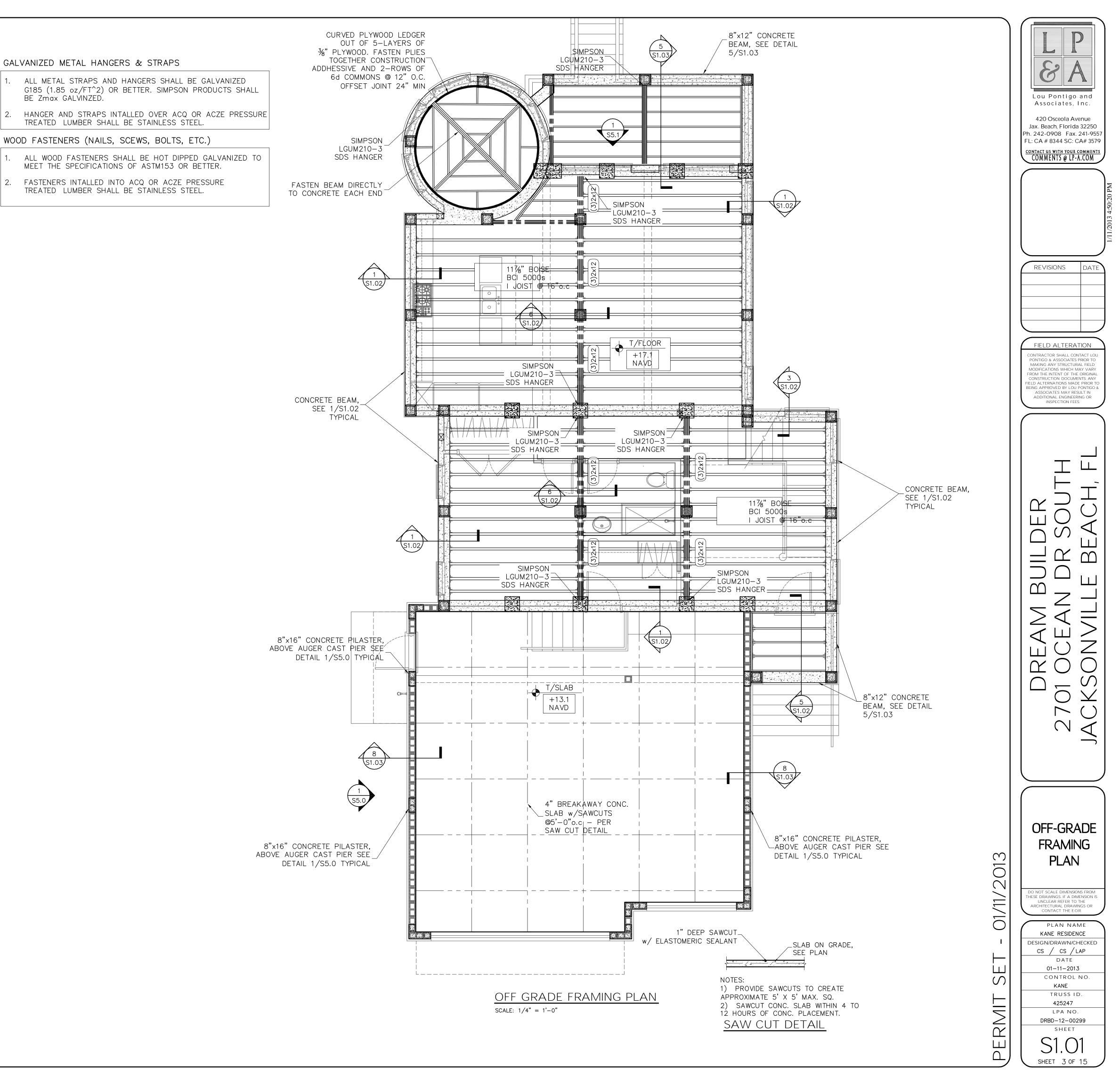
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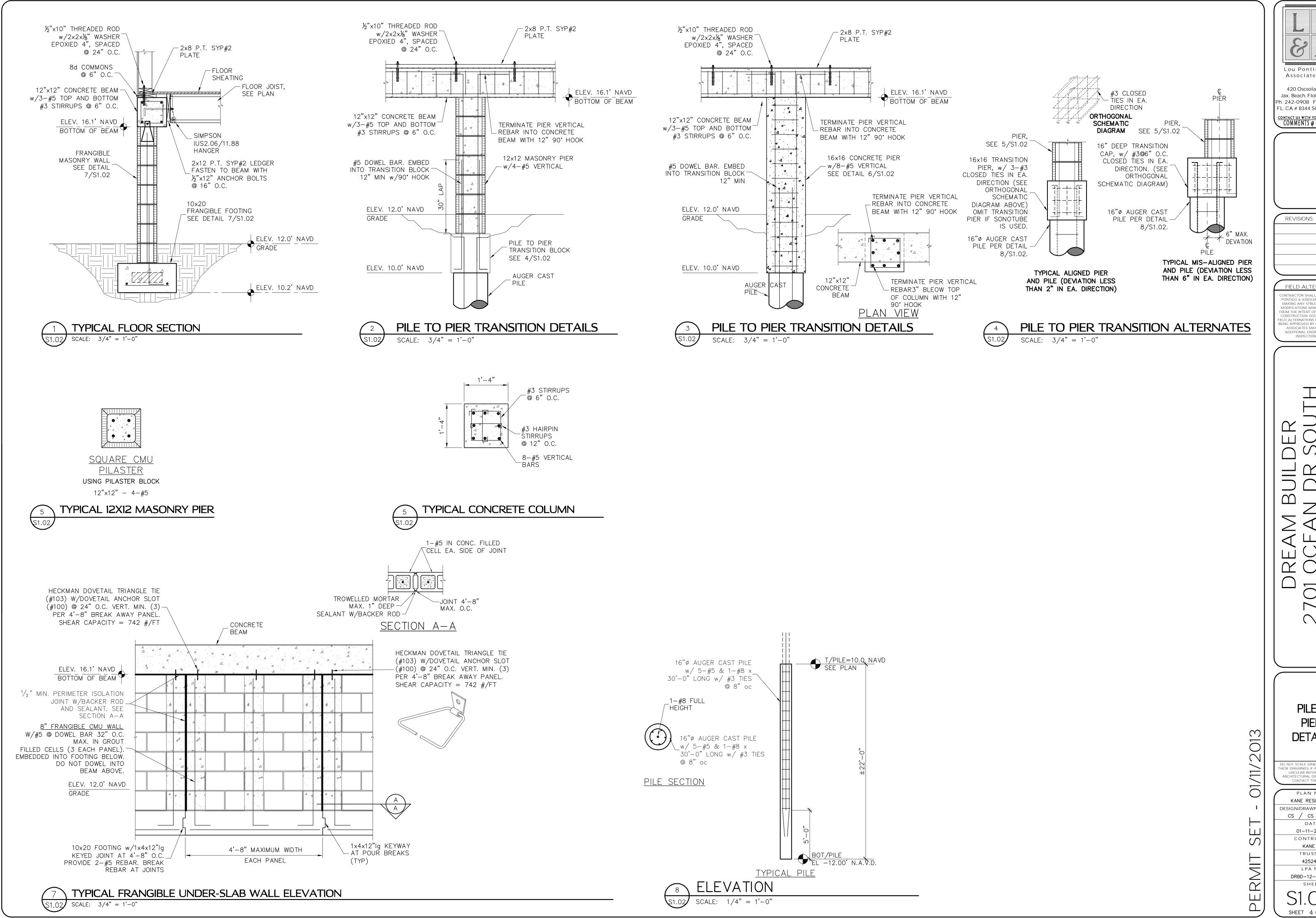
SHEET

SHEET 2 OF 15

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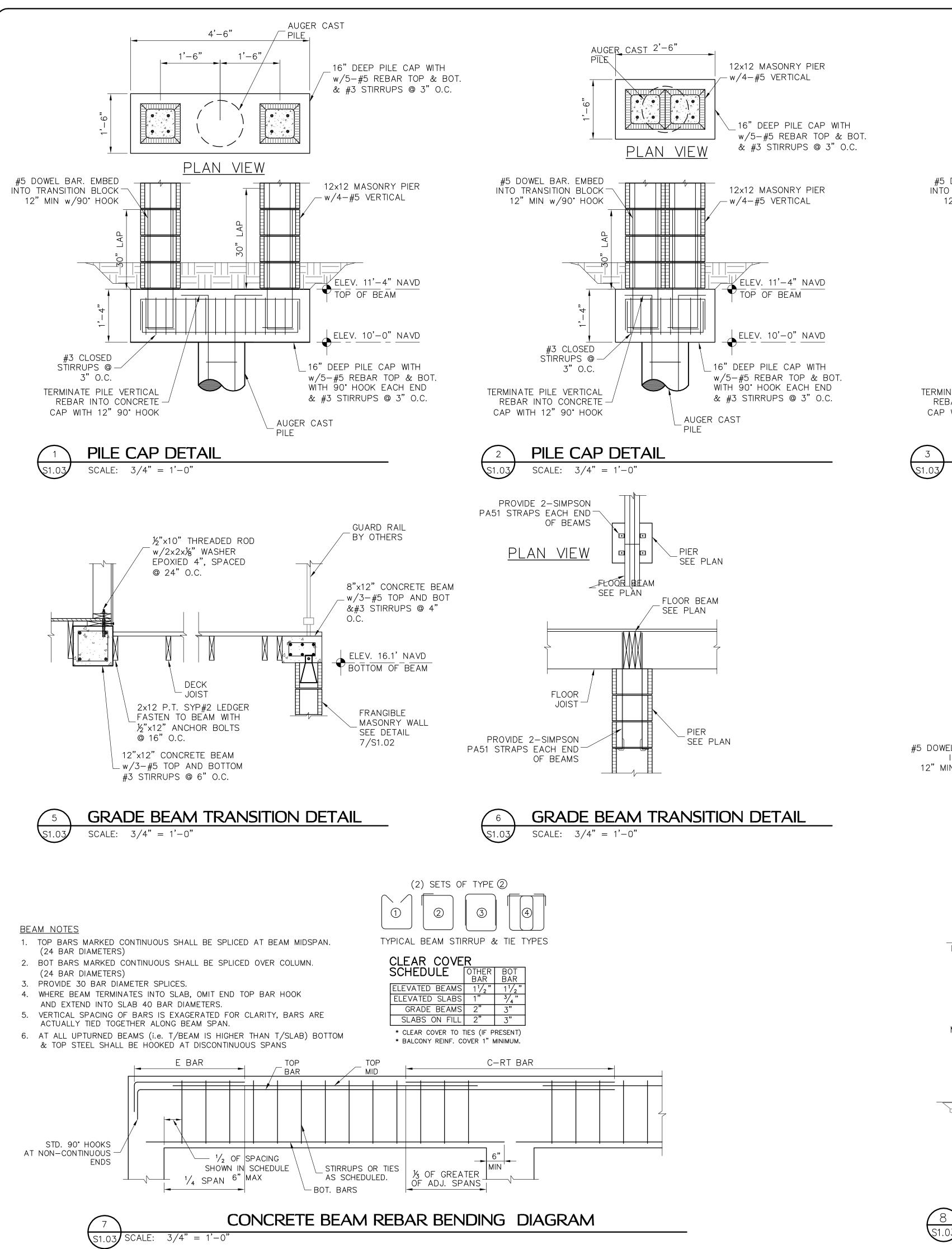
PILE & PIER **DETAILS** 

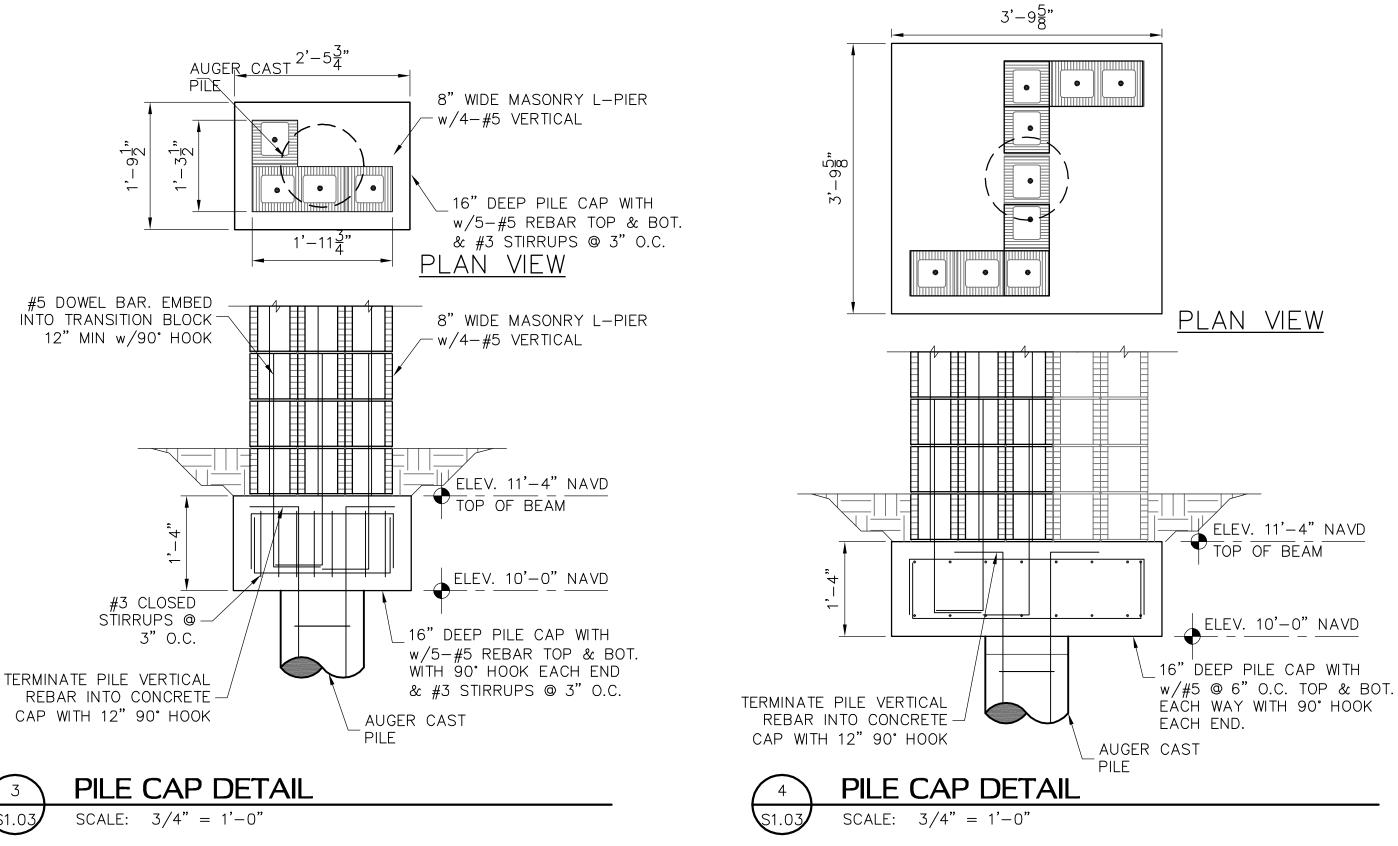
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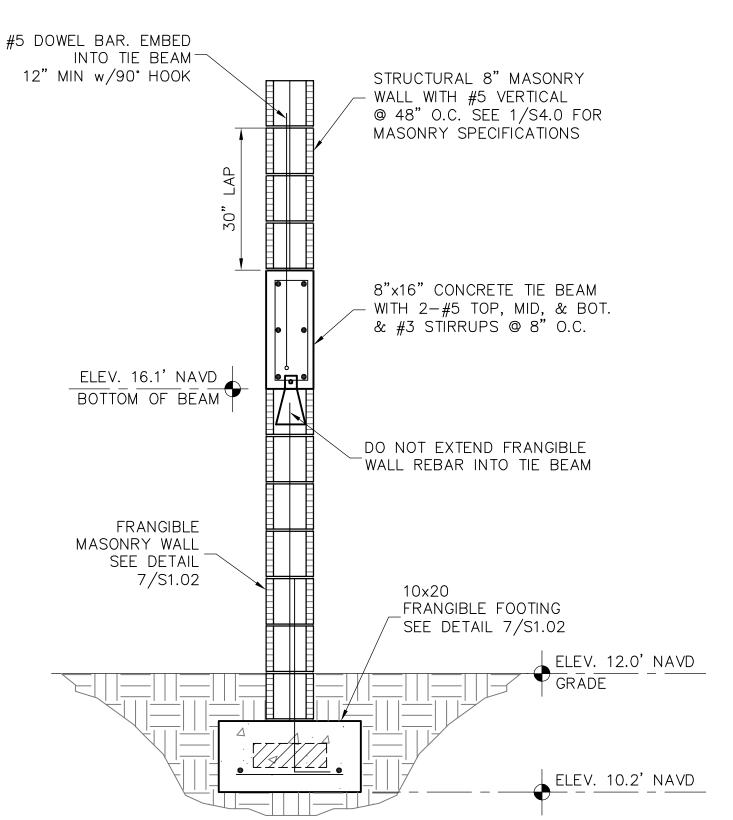
KANE RESIDENCE DESIGN/DRAWN/CHECKED CS / CS / LAP 01-11-2013

CONTROL NO. KANE TRUSS ID. 425247 LPA NO. DRBD-12-00299

SHEET SHEET 4 OF 12









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PILE CAP & CONCRETE **BEAM DETAILS** 

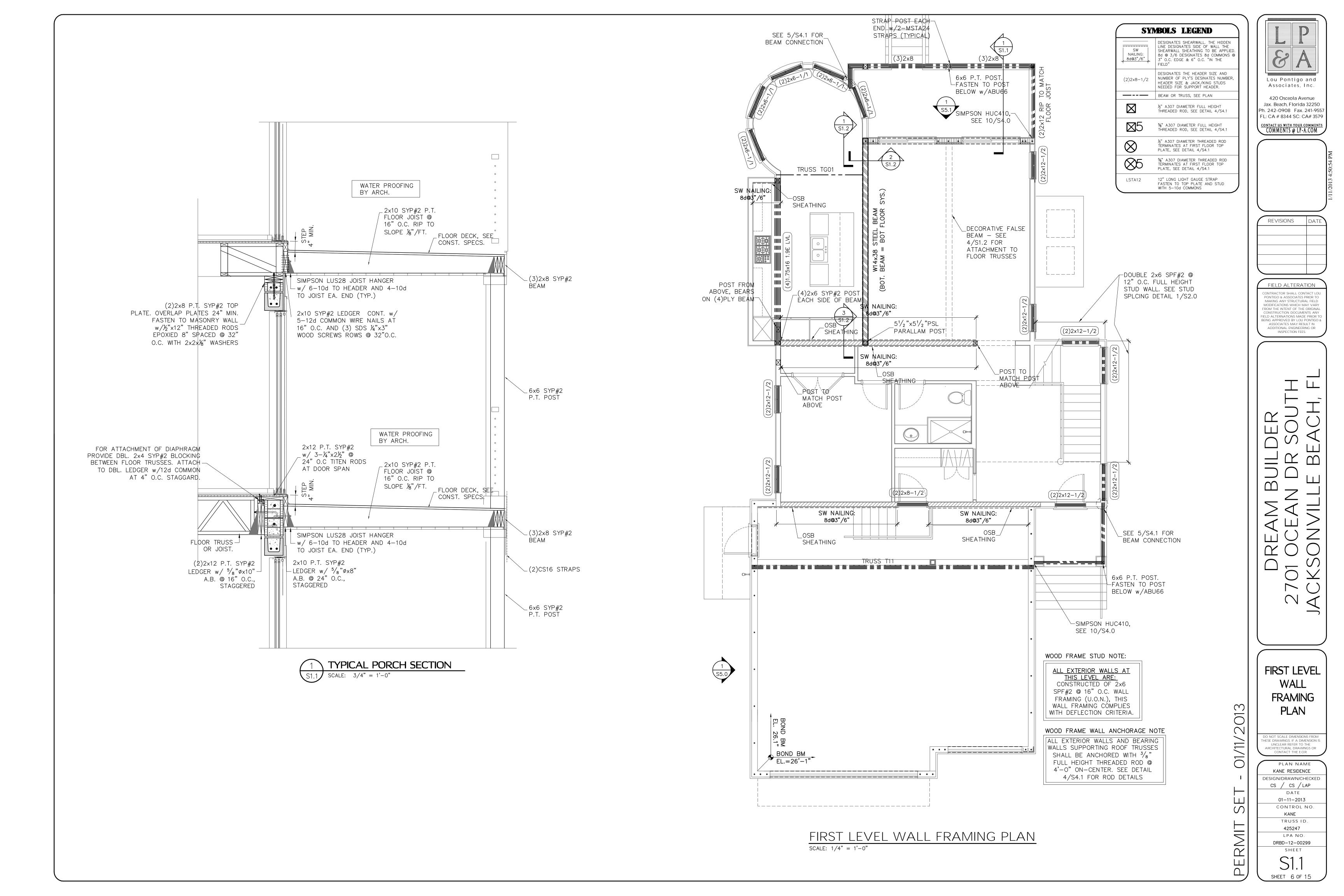
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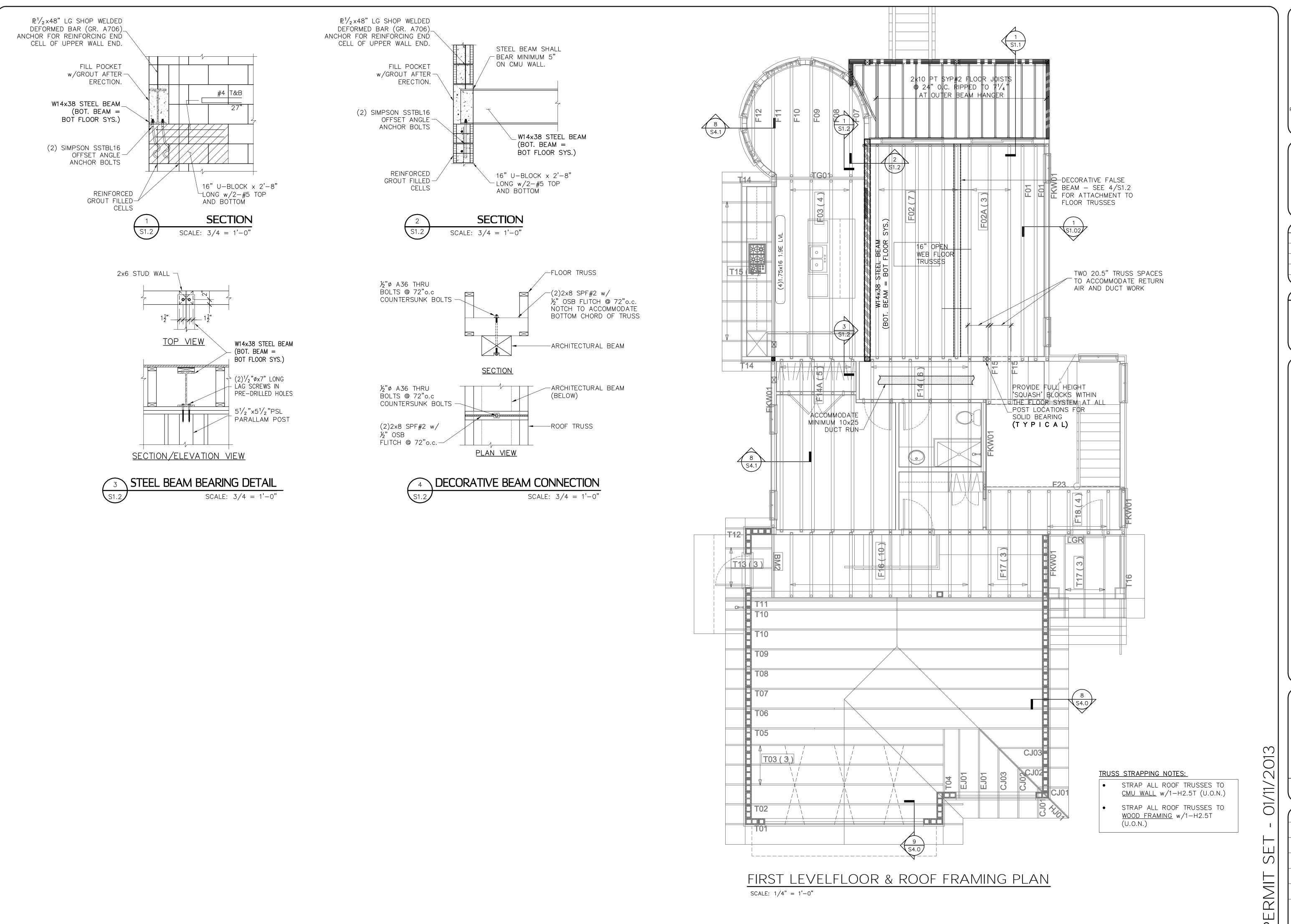
PLAN NAME KANE RESIDENCE DESIGN/DRAWN/CHECKED CS / CS / LAP 01-11-2013

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SHEET

S1.03 SHEET 5 OF 15





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FIRST LEVEL FLOOR & **ROOF FRAMING** PLAN

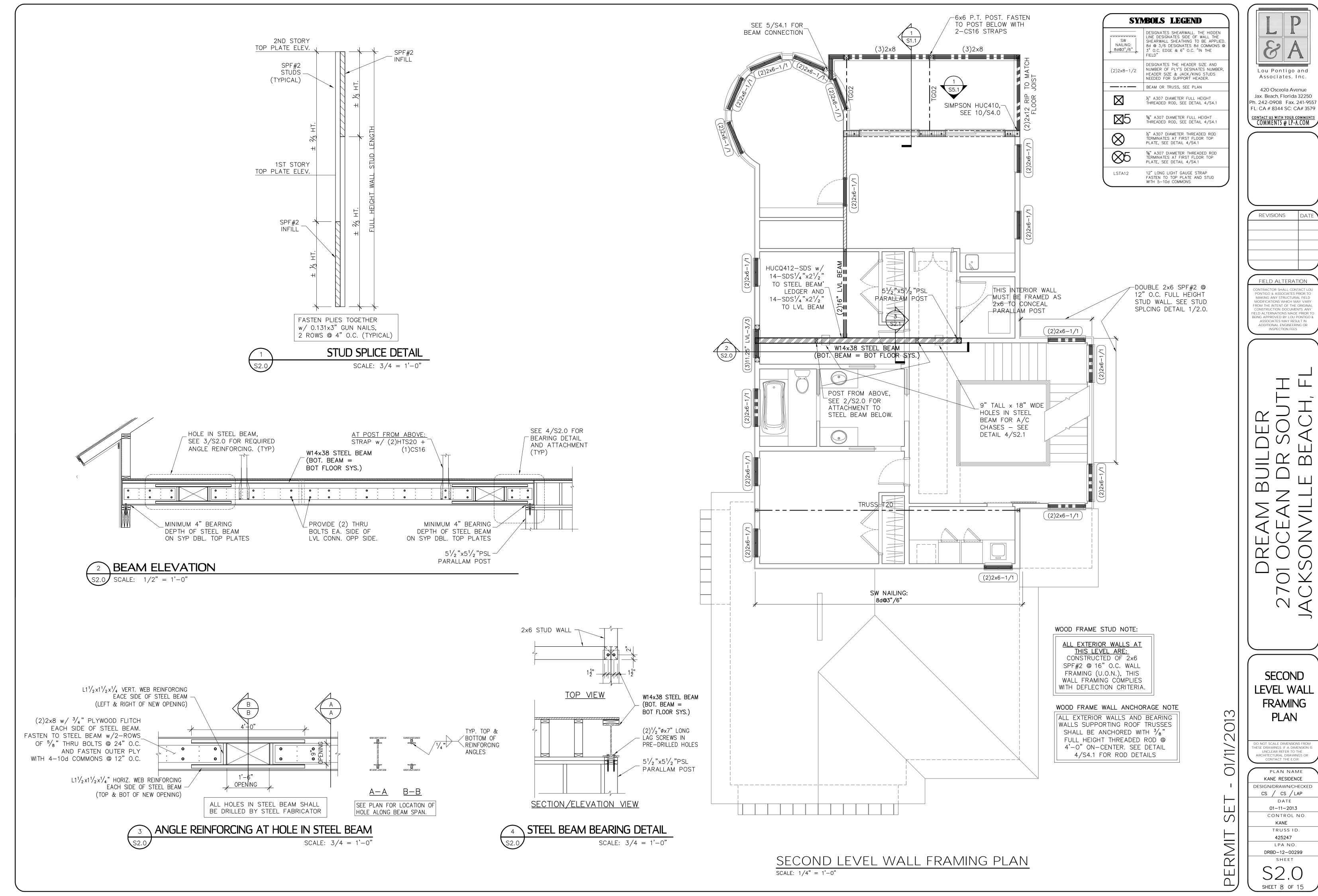
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**SECOND** LEVEL WALL **FRAMING PLAN** 

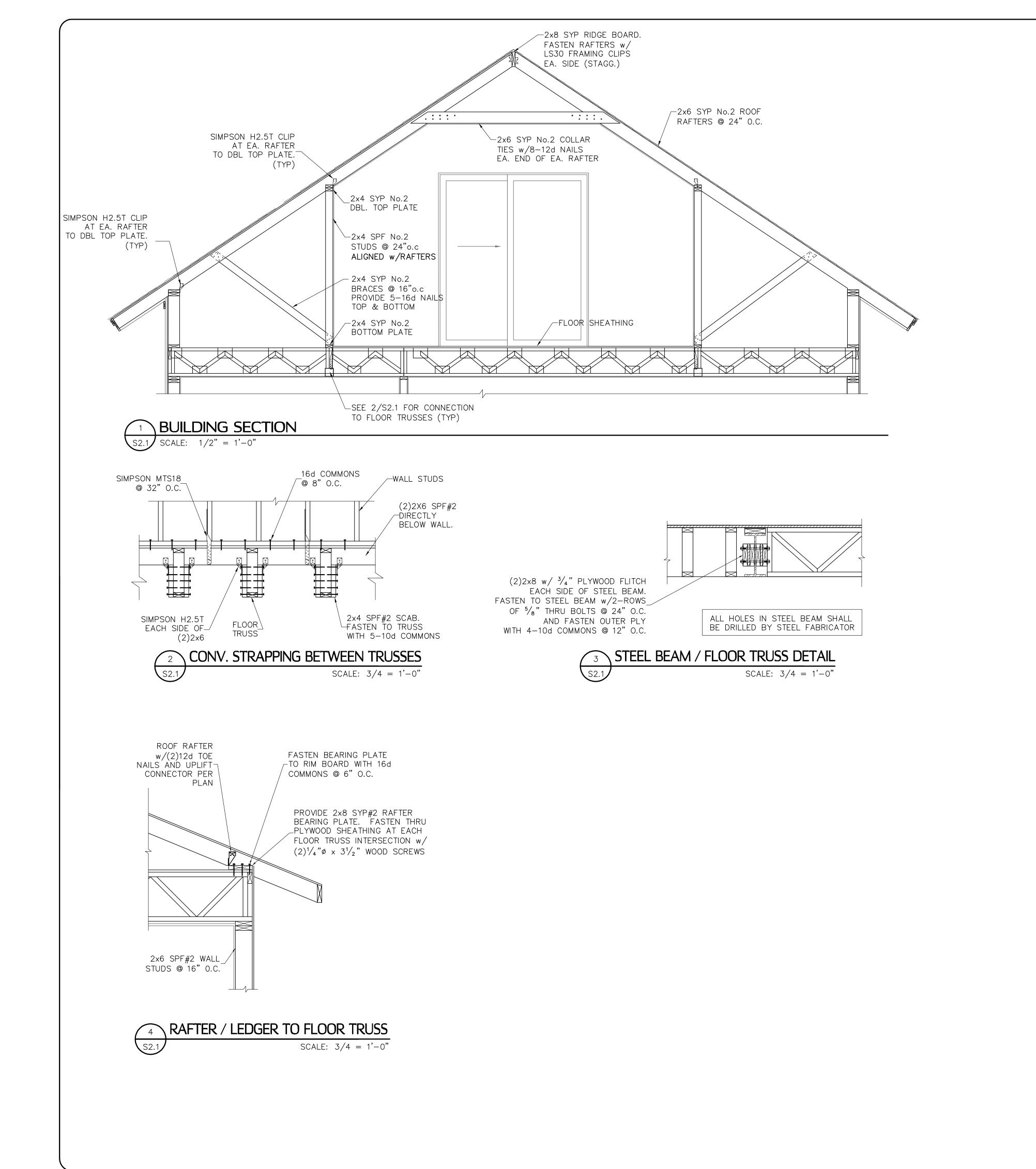
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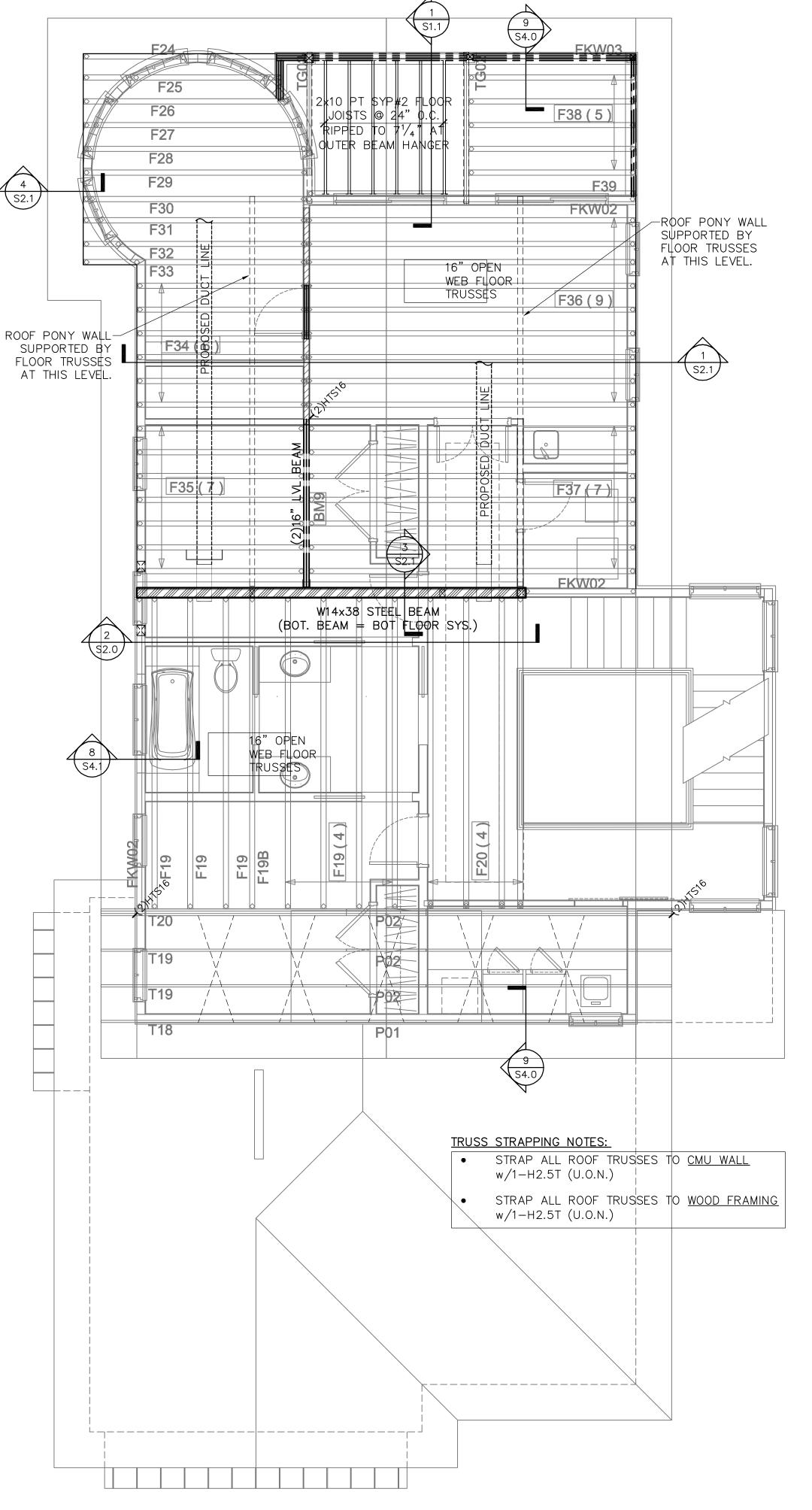
PLAN NAME KANE RESIDENCE DESIGN/DRAWN/CHECKED CS / CS / LAP 01-11-2013

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SHEET 8 OF 15





3 SET - 01/11/

SECOND LEVEL FLOOR FRAMING PLAN

SCALE: 1/4" = 1'-0"

SHEET SHEET 9 OF 15

**SECOND** 

**LEVEL** 

**FLOOR** 

**FRAMING** 

PLAN

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DESIGN/DRAWN/CHECKED

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KANE

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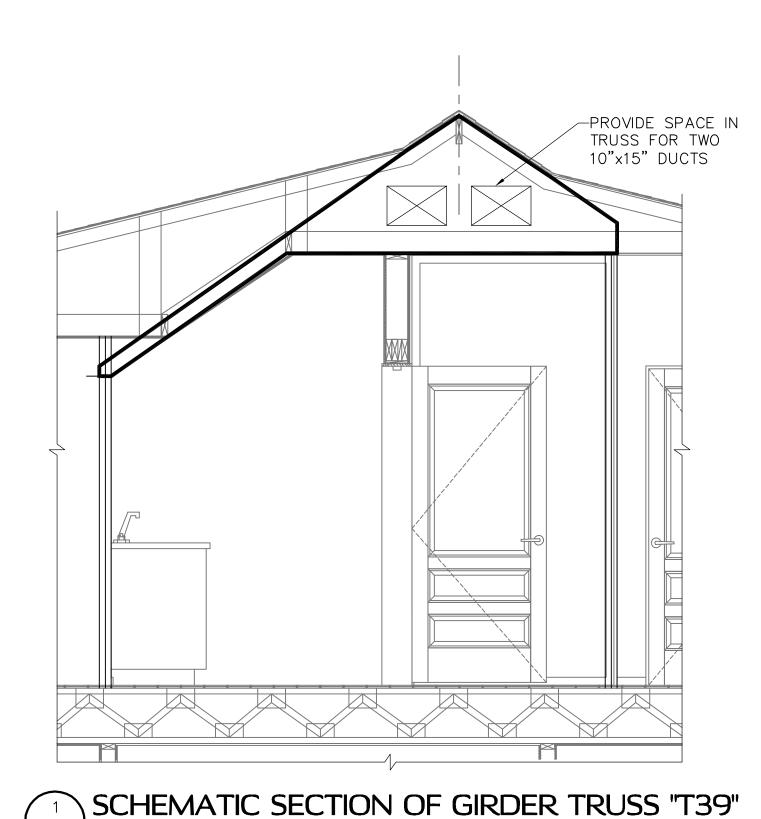
FROM THE INTENT OF THE ORIGINAL CONSTRUCTION DOCUMENTS. ANY

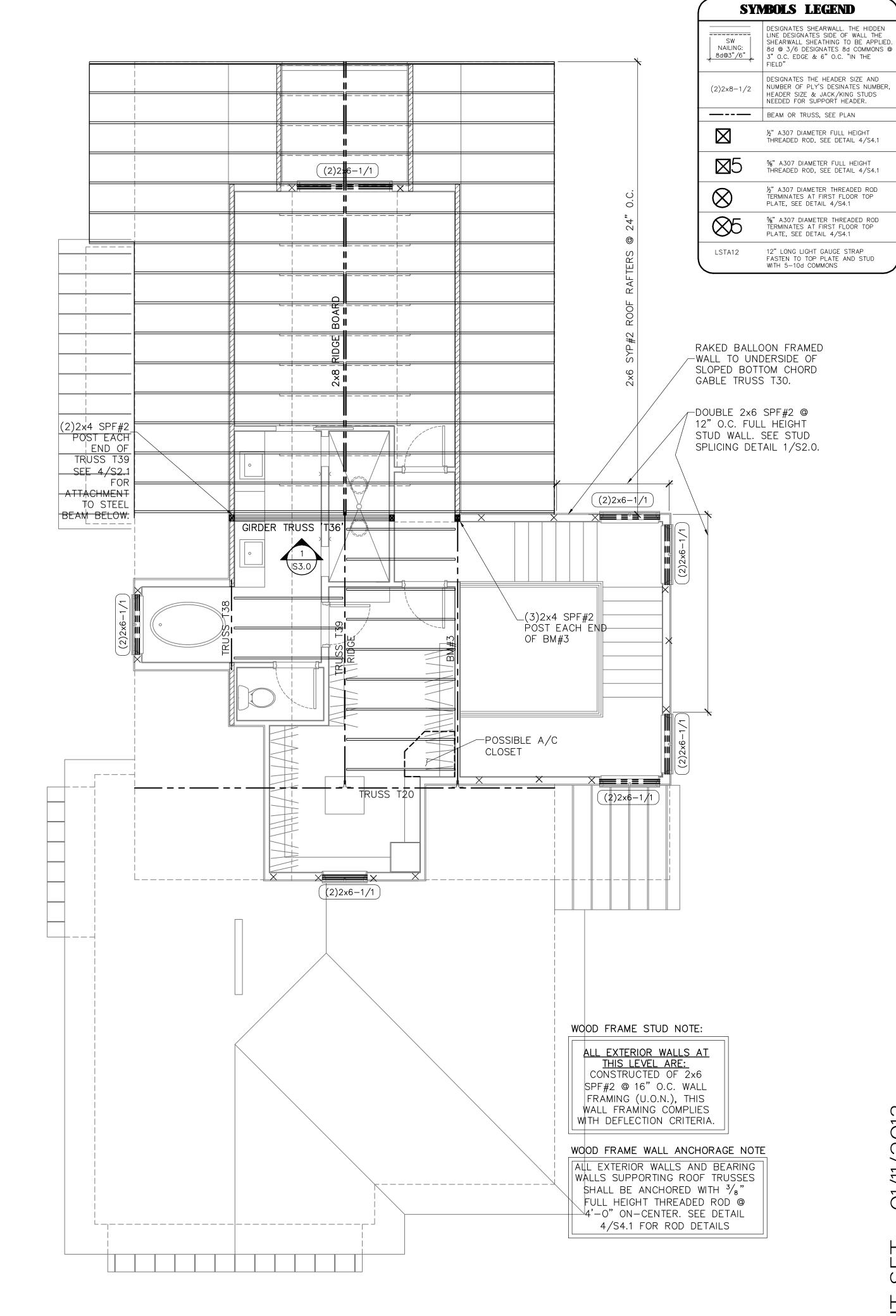
Being Approved by Lou Pontigo Associates May Result in Additional Engineering or Inspection Fees.

LD ALTERNATIONS MADE PRIOR

 $\Box$ 

Associates, Inc.





THIRD LEVEL WALL FRAMING PLAN SCALE: 1/4" = 1'-0"

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THIRD LEVEL WALL FRAMING PLAN

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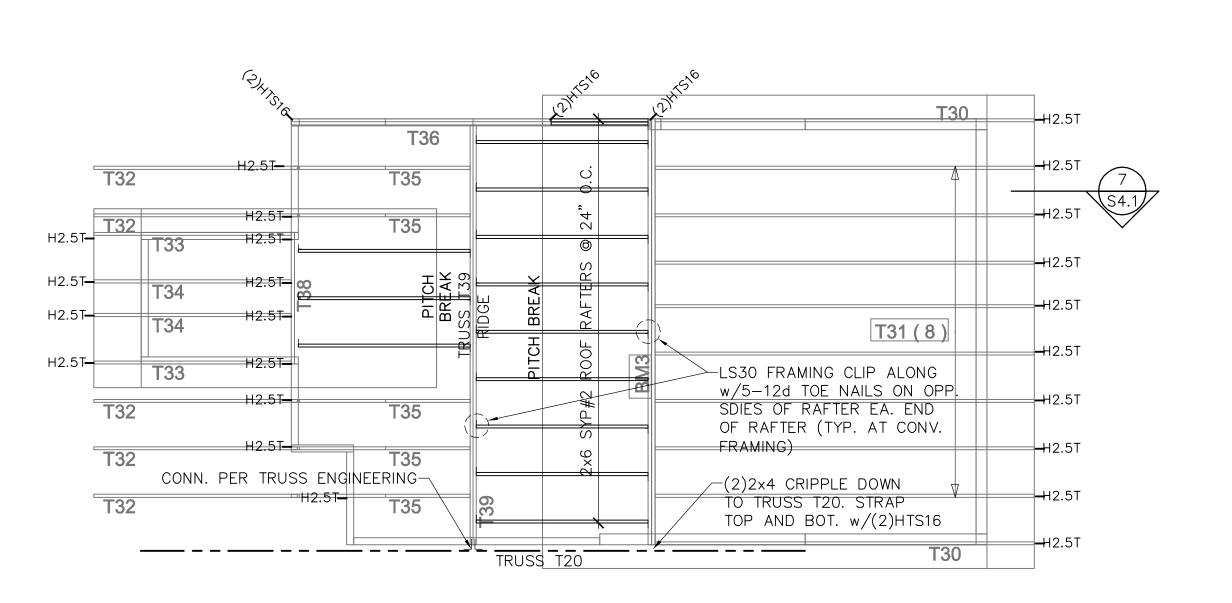
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THIRD LEVEL ROOF FRAMING PLAN

SCALE: 1/4" = 1'-0"



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THIRD LEVEL ROOF **FRAMING** PLAN

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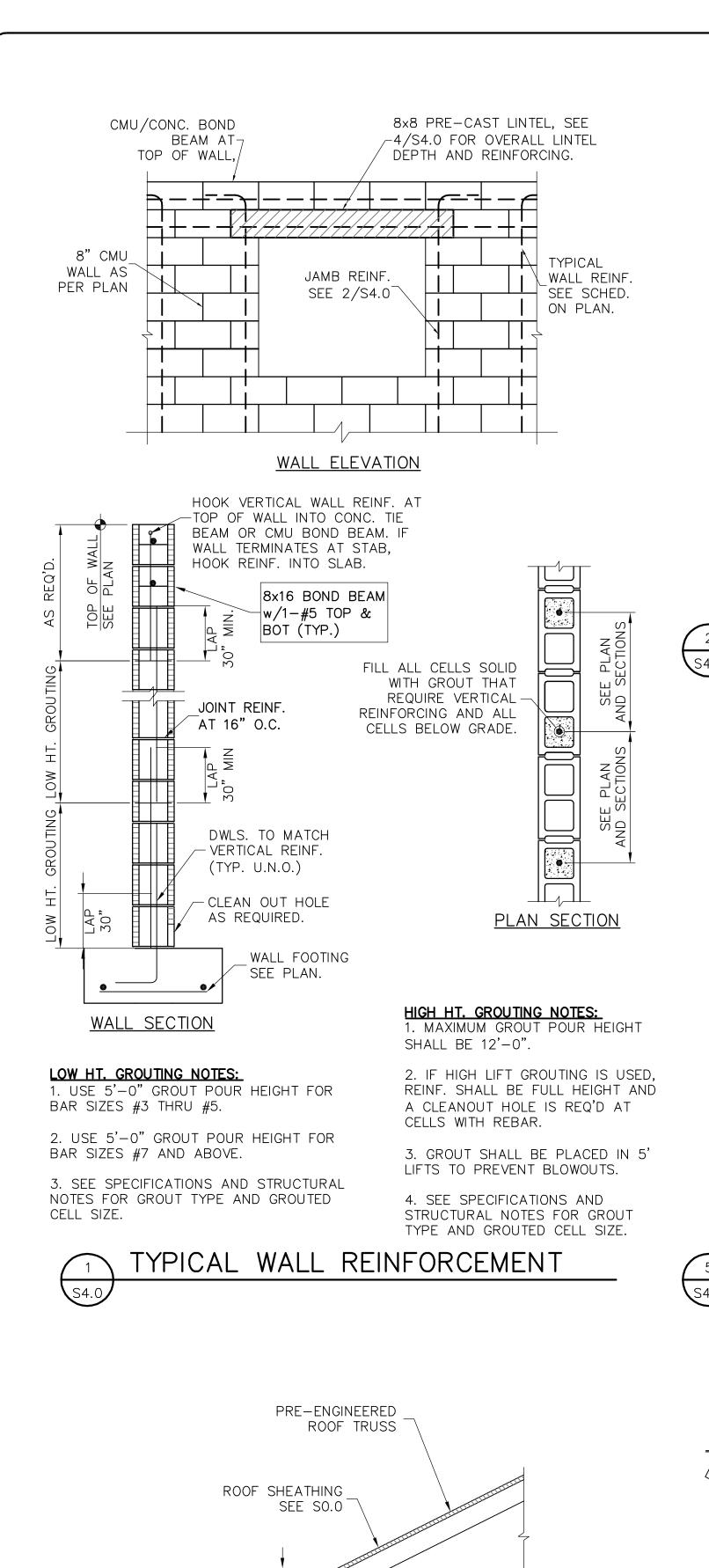
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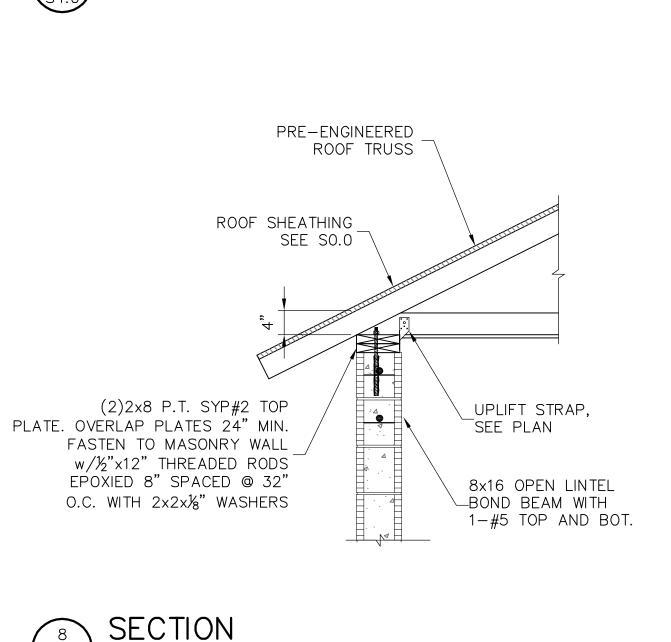
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TRUSS ID. 425247 DRBD-12-00299

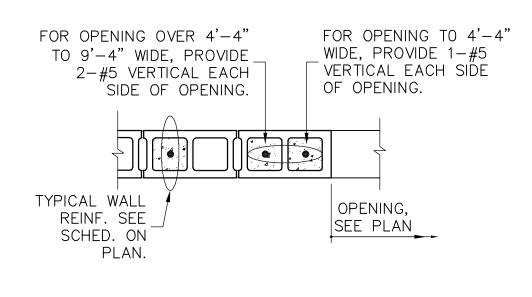
SHEET S3.1

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SCALE: 3/4" = 1'-0"



PLAN VIEW AT TYPICAL WINDOW/DOOR OPENING

CMU JAMB REINFORCING

SCALE: 3/4" = 1'-0"

2x4 SYP#2 CATS AT 12"

O.C. FASTENED TO TOP-

PLATE w/4-0.131x3"

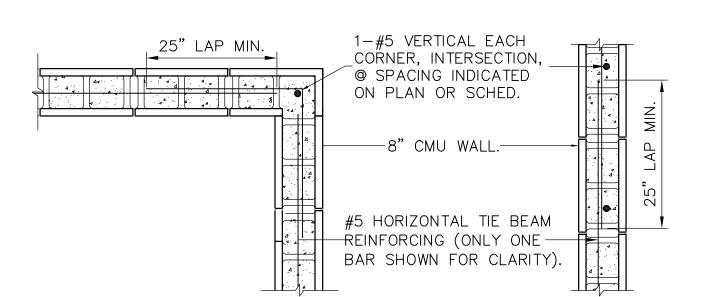
FASTEN CATS EACH

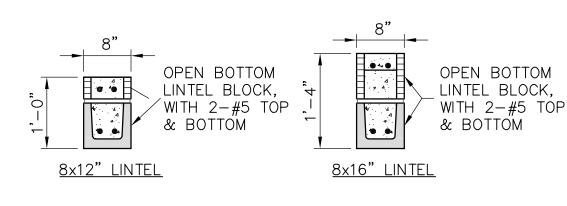
END  $w/3-0.131\times3$ "

COMMÓN TOE NAILS

SECTION

COMMON.

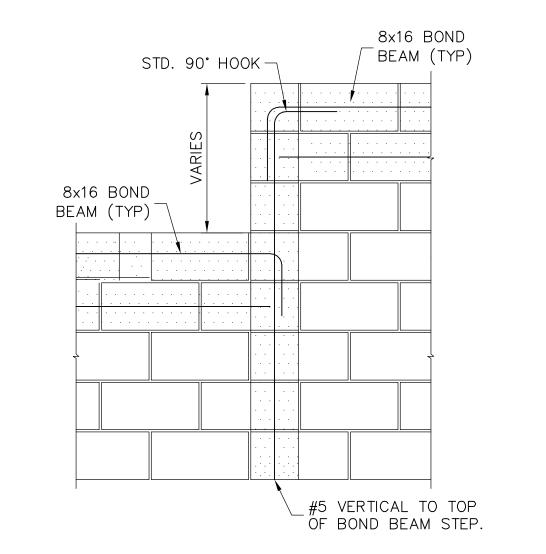


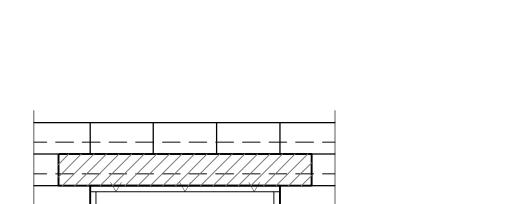


 USE THESE LINTEL DETAILS U.O.N. ON PLANS. FOR OPENING LOCATIONS, SEE ARCH. PLANS. PROVIDE 8" BEARING EACH SIDE OF OPENING

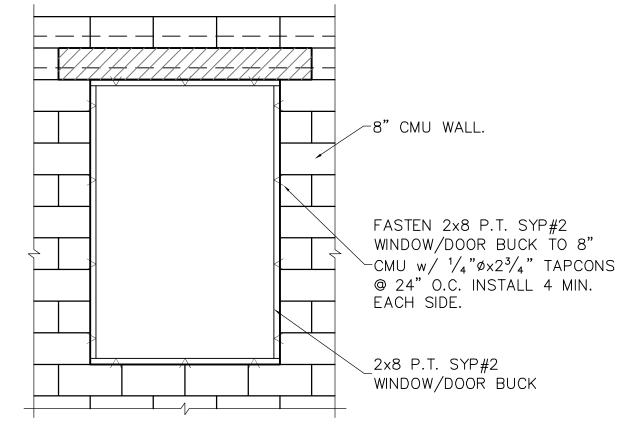
GROUT LINTEL SOLID. BOTTOM SHALL BE A PRECAST "U" LINTEL.

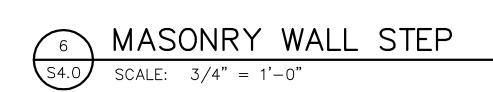
**TYPICAL** HORZONTAL/CORNER SPLICE TYPICAL CMU LINTEL





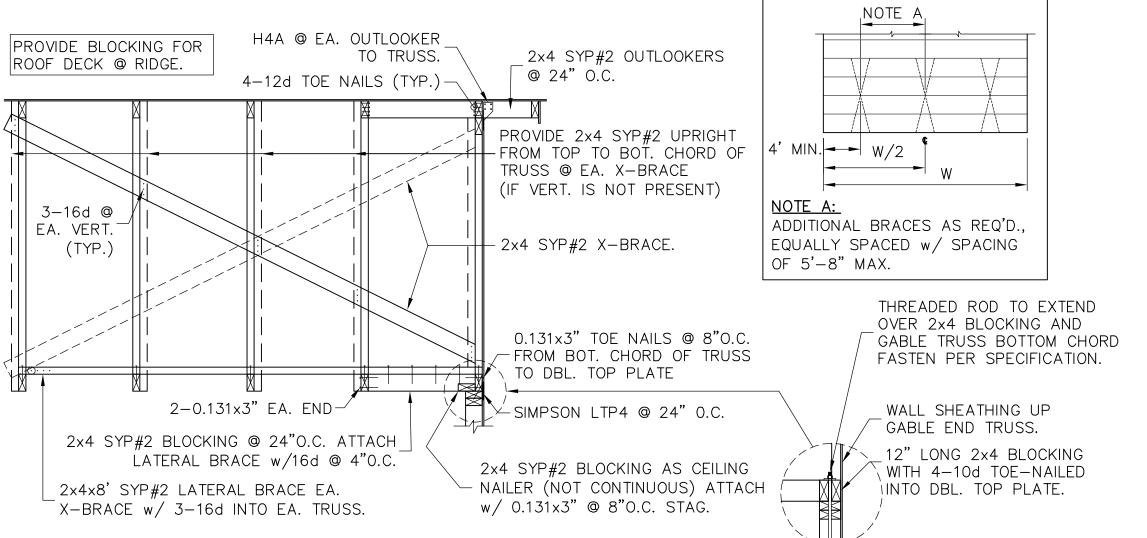
SCALE: 3/4" = 1'-0"





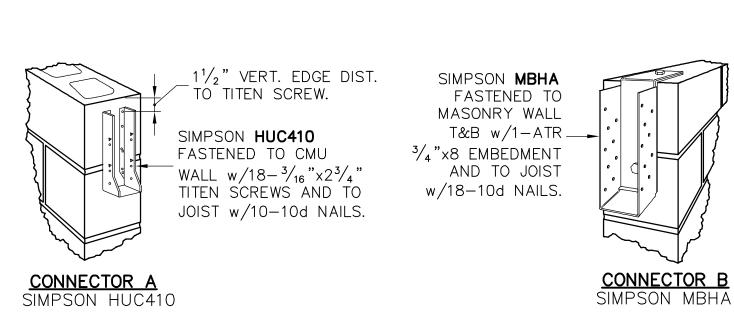
X-BRACE SPACING SCHEMATIC

SCALE: 3/4" = 1'-0"



TRUSS.

~CMU WALL



MASONRY DOOR/WINDOW

BUCK DETAIL

SCALE: 3/4" = 1'-0"

1. CONNECTORS TO BE ATTACHED TO FULLY GROUTED CMU WALL.

2. ATR IS ALL THREADED ROD.

3. MBHA SHALL BE INSTALLED W/ATR USING SIMPSON SET EPOXY-TIE  $6\frac{3}{4}$ " MIN. EMBEDMENT REQUIRED. ATR TO BE  $\frac{3}{4}$ " x 8" LONG, GRADE A307 OR BETTER.

4. SKEWED CONNECTOR OPTION IS AVAILABLE FOR BOTH HUC410 AND MBHA. INSTALLATION IS SIMILAR TO DETAILS ABOVE.



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**MASONRY DETAILS** 

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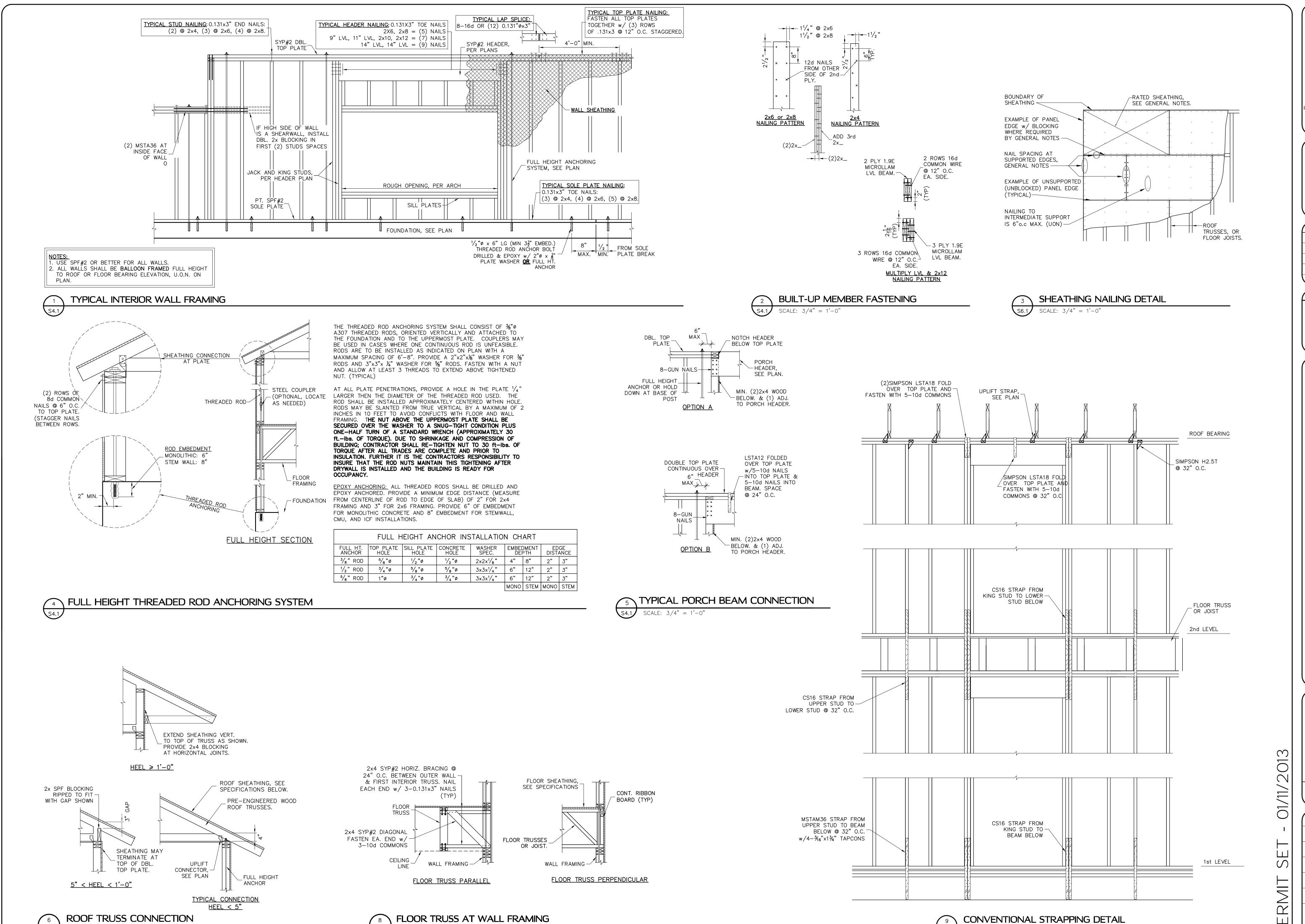
01-11-2013 CONTROL NO. KANE TRUSS ID. 425247 LPA NO.

SHEET 12 0F15

SHEET S4.0

DRBD-12-00299

GABLE END BRACING SCALE: 3/4" = 1'-0"



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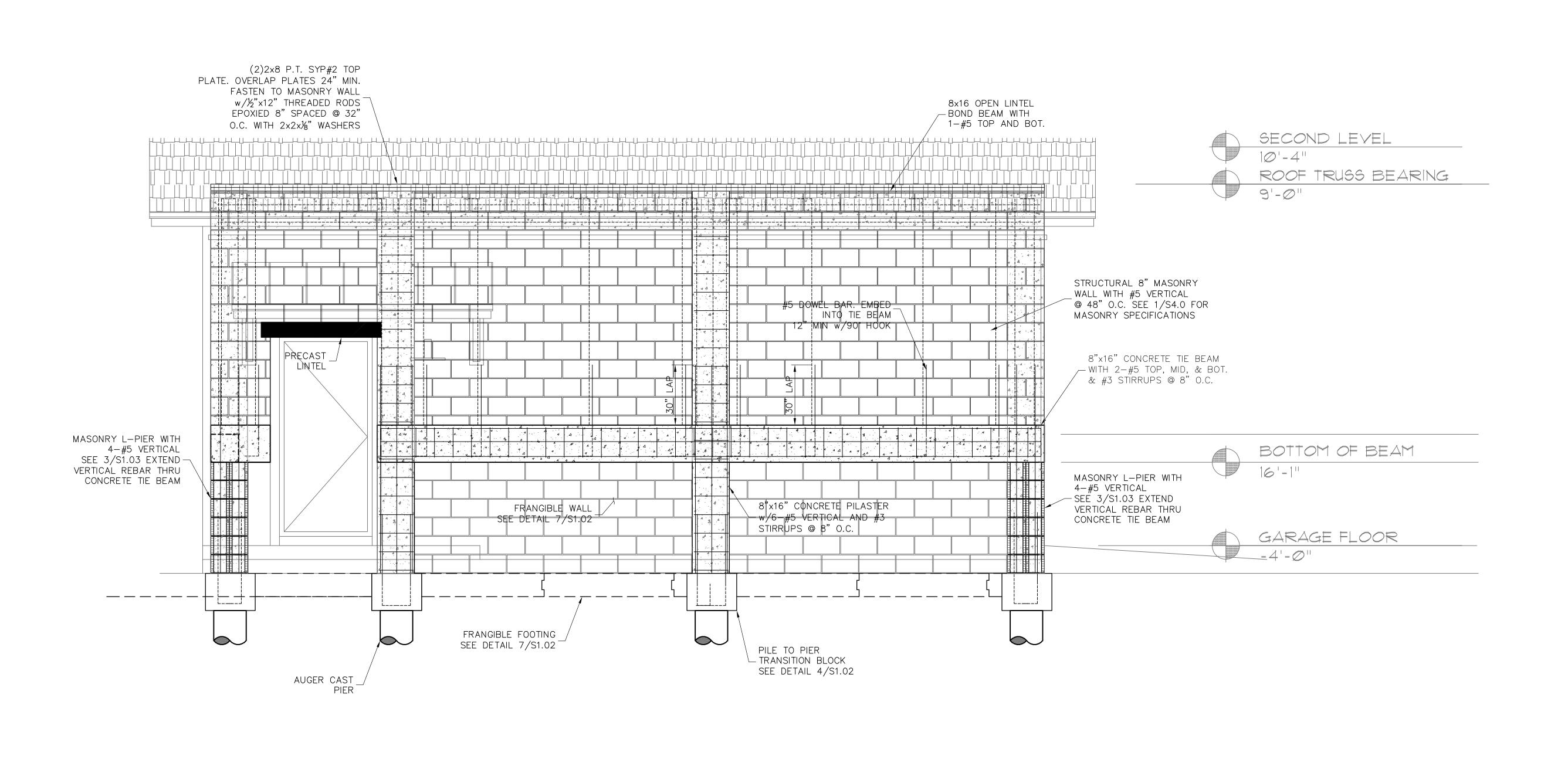
 $\mathbf{\Omega}$ 

FRAMING & **TIE-DOWN DETAILS** 

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> LPA NO. DRBD-12-00299 SHEET SHEET 130F15



GARAGE WALL ELEVATION

DREAM BUILDEF 2701 OCEAN DR SO ACKSONVILLE BEAC

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PLAN NAME
KANE RESIDENCE

DESIGN/DRAWN/CHECKED

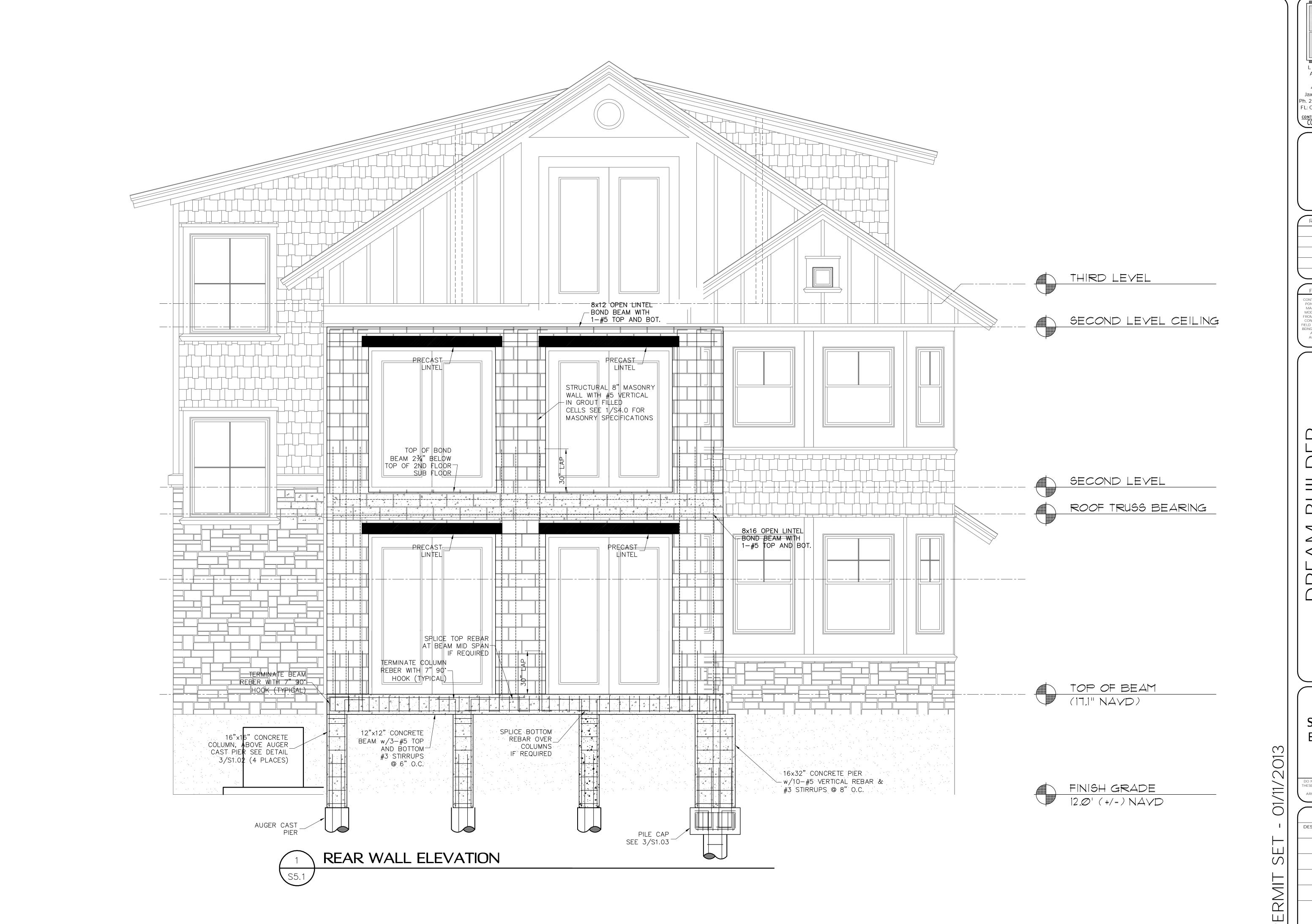
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DATE
01-11-2013
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KANE
TRUSS ID.
425247

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SHEET

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